

**TABLICE I DIJAGRAMI**  
iz predmeta  
**BETONSKE KONSTRUKCIJE II**

**TABLICA 1: PARCIJALNI KOEFICIJENTI SIGURNOSTI ZA  
DJELOVANJA**

Djelovanje	Parcijalni koeficijenti sigurnosti $\gamma_F$		
	Vrsta djelovanja		
	Stalno $\gamma_G$	Promjenjivo $\gamma_Q$	Prednapinjanje $\gamma_P$
Nepovoljno	1.35	1.5	1.0 ili 1.1
Povoljno	1.0	0	1.0 ili 0.9

**TABLICA 2: KARAKTERISTIČNE VRIJEDNOSTI KORISNOGA  
OPTEREĆENJA ZA ZGRADE RAZLIČITIH KATEGORIJA**

<b>Kategorija objekta</b>	Vertikalno promjenjivo opterećenje
	<b><math>q_k</math></b> [kN/m <sup>2</sup> ]
<b>A-</b> stambene prostorije, sobe u zgradama, sobe u bolnicama i hotelima, kuhinje, toaleti..... - stubišta..... - balkoni.....	2.00 3.00 4.00
<b>B-</b> poslovne prostorije.....	3.00
<b>C-</b> prostorije za okupljanje ljudi	
<b>C<sub>1</sub></b> - prostorije sa stolovima (škole, restorani, čitaonice).....	3.00
<b>C<sub>2</sub></b> - prostorije s fiksnim sjedištim (kina, kazališta, čekaonice).....	4.00
<b>C<sub>3</sub></b> - prostorije bez ograničenja kretanja ljudi muzeji, pristupne površine u javnim administrativnim zgradama i hotelima).....	5.00
<b>C<sub>4</sub></b> - prostorije za posebne fizičke aktivnosti (prostорије за пles, gimnastičке дворане, бине).....	5.00
<b>C<sub>5</sub></b> - površine s izrazitim ljudskim okupljanjem (koncertne дворане, sportske дворане са стајањем, терасе и сл.).....	5.00
<b>D-</b> prodavaonice	
<b>D<sub>1</sub></b> - maloprodajne trgovine.....	5.00
<b>D<sub>2</sub></b> - robne kuće, skladišta, trgovine kancelarijskog materijala.....	5.00
<b>E-</b> skladišta i biblioteke.....	min 6.00
<b>F-</b> prometne i parkirne površine za laka vozila ukupne težine $\leq 30\text{kN}$ i s $\leq 8$ sjedišta.....	2.00
<b>G-</b> za vozila srednje ukupne težine od 30 do 160 kN na dvije osovine.....	5.00

<b>Kategorije krova</b>	Nagib krova	<b><math>q_k</math></b> [kN/m <sup>2</sup> ]
Kategorija H	< 20°	0.75
	> 40°	0.00

**TABLICA 3. KOEFICIJENTI KOMBINACIJE**

Koeficijenti kombinacije			
Djelovanje	$\Psi_0$	$\Psi_1$	$\Psi_2$
<b>Pokretno opterećenje na ploče</b>	<b>0.7</b>	<b>0.5</b>	<b>0.3</b>
- stanovi, uredi, trgovine do 50m <sup>2</sup> , predvorja, balkoni, bolnice			
- prostor za skupove, garaže, zgrade za parkiranje, gimnastičke dvorane, predvorja učionica, knjižnice, arhivi	<b>0.8</b>	<b>0.8</b>	<b>0.5</b>
- prostor za izložbe i trgovinu, trgovačke i robne kuće	<b>0.8</b>	<b>0.8</b>	<b>0.8</b>
<b>Vjetar</b>	<b>0.6</b>	<b>0.5</b>	<b>0</b>
<b>Snijeg</b>	<b>0.7</b>	<b>0.2</b>	<b>0</b>
<b>Sva druga djelovanja</b>	<b>0.8</b>	<b>0.7</b>	<b>0.5</b>

Za osnovnu kombinaciju računske se veličine reznih sila proračunavaju po izrazu:

$$S_d = S_d \left[ \sum_j (\gamma_{G,j} \cdot G_{k,j}) + \gamma_Q \cdot Q_{k,1} + \sum_{i>1} (\gamma_Q \cdot \Psi_{0,i} \cdot Q_{k,i}) + \gamma_p \cdot P_k \right] \quad (6.3)$$

gdje su:

$\gamma_G, \gamma_Q, \gamma_p$  - parcijalni koeficijenti sigurnosti za djelovanje dani u tablici 6.3.

$G_{k,j}, Q_{k,i}$  - karakteristične veličine za stalno i promjenjivo opterećenje (djelovanje)

$Q_{k,1}$  - karakteristična veličina nepovoljnog jedinog ili vodećeg promjenjivog djelovanja kad istodobno djeluje više promjenjivih opterećenja

$P_k$  - karakteristična veličina prednapinjanja

$\Psi_{0,i}$  - koeficijenti kombinacije za promjenjiva djelovanja dani u tablici 6.2.

**TABLICA 4. PARCIJALNI KOEFICIJENTI SIGURNOSTI ZA MATERIJALE**

<b>Vrsta kombinacije</b>	<b>Parcijalni koeficijenti sigurnosti <math>\gamma_m</math></b>	
	<b>Beton <math>\gamma_c</math></b>	<b>Betonski i prednapeti čelik <math>\gamma_s</math></b>
<b>Osnovna kombinacija</b>	<b>1.50</b>	<b>1.15</b>
<b>Slučajna kombinacija</b>	<b>1.30</b>	<b>1.00</b>

Parcijalnim koeficijentima sigurnosti za materijale  $\gamma_M$  obuhvaća se:

- razlika u čvrstoći gradiva dobivena preko uzorka i one u konstrukciji;
- moguće lokalno slabljenje materijala ili konstrukcije za vrijeme građenja;
- eventualne netočnosti u određivanju otpornosti elementa na osnovi otpornosti gradiva.

**TABLICA 5. KLASE I OPISI OKOLIŠA**

Klasa okoliša	Primjeri za navedeni okoliš	
1 Suhı okoliš	Unutrašnji elementi zgrade	
2 Vlažni okoliš	a <sup>2)</sup>	Unutrašnji elementi u jako vlažnom okolišu Vanjski elementi Elementi koji nisu u agresivnom tlu ili/i vodi
	b <sup>3)</sup>	Kao 2a, ali elementi izloženi povremeno smrzavici
3 Vlažni okoliš, smrzavanje, odmrzavanje	Elementi izloženi vremenskim nepogodama, smrzavanje, odmrzavanje solju	
4 Blizina mora	a	Elementi izloženi kapljicama mora ili djelomično uronjeni u more
	b	Kao 4a, ali elementi izloženi mrazu
5 Kemijski agresivan okoliš <sup>1)</sup>	a	Slabo kemijski agresivan okoliš, agresivna atmosfera (industrija)
	b	Umjерено kemijski agresivan okoliš
	c	Jako kemijski agresivan okoliš

- 1) Kemijski agresivan okoliš posebno je klasificiran preko  
internacionalne organizacije za standardizaciju (ISO),  
2) bez smrzavanja,  
3) sa smrzavanjem.

**TABLICA 6. DEBLJINE ZAŠTITNIH SLOJEVA U OVISNOSTI O KLASI OKOLIŠA**

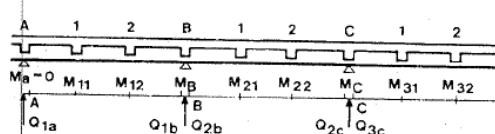
		Klasa okoliša								
		1	2a	2b	3	4a	4b	5a	5b	5c
Minimalna debljina zaštitnog sloja "c" (mm)	Betonski čelik	15	25	25	40	40	40	25	30	40
	Prednapeti čelik	25	30	35	50	50	50	35	40	50

Najmanje zaštitne slojeve valja korigirati i to:

- povećati za 0.5 cm ako su površine betona teško dostupne kontroli;
- povećati za 0.5 cm za marke betona manje od  $25 \text{ N/mm}^2$ ;
- povećati za 1.0 cm kada se površine betona naknadno obrađuju;
- povećati za 1.0 cm za konstrukcije koje se izvode klizajućom oplatom;
- smanjiti za 0.5 cm za montažne elemente proizvedene u tvorničkim uvjetima.

## TABLICA 7. PRORAČUN MOMENATA SAVIJANJA I POPREČNIH SILA KONTINUIRANIH NOSAČA

Nosači sa 2 do 5 jednakih polja s jednakim rasporedom opterećenja u poljima (kontinuiranim ili koncentriranim)



Nosač preko dva jednaka raspona

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju								
		$x = (0.4 \pm 0.5)l$	$\frac{P}{2}$	$\frac{P}{2}$	$\frac{P}{3} / \frac{1}{1/3}$	$\frac{P}{4} / \frac{1}{1/2} / \frac{P}{4}$	$\frac{P}{4} / \frac{1}{1/4} / \frac{P}{4}$	$\frac{P}{6} / \frac{1}{1/3} / \frac{1}{1/3}$	$\frac{1}{2} / \frac{1}{2}$	$0.4 / 0.2 / 0.4 / 0.4$
0.2001 0.2001	M <sub>11</sub>	0.070 pl <sup>2</sup>	0.156 F	0.222 F	0.180 F	0.258 F	0.184 F	0.095 K	0.094 K	0.089 K
	M <sub>12</sub>	-	-	0.111 F	0.039 F	0.266 F	0.219 F	-	-	-
	M <sub>13</sub>	-	-	-	-	0.023 F	-0.080 F	-	-	-
Za stalno opterećenje stavlja se g umjesto P, odnosno g, umjesto p	M <sub>Bmin</sub>	-0.125 pl <sup>2</sup>	-0.188 F	-0.333 F	-0.281 F	-0.469 F	-0.396 F	-0.156 K	-0.155 K	-0.151 K
	R <sub>A</sub> =Q <sub>1A</sub>	0.375 pl	0.313 F	0.667 F	0.719 F	1.031 F	1.104 F	0.344 K	0.345 K	0.349 K
	R <sub>Bmax</sub>	1.250 pl	1.375 F	2.667 F	2.563 F	3.938 F	3.792 F	1.312 K	1.310 K	1.302 K
	Q <sub>1Bmin</sub>	-0.625 pl	-0.688 F	-1.333 F	-1.281 F	-1.969 F	-1.896 F	-0.656 K	-0.655 K	-0.651 K
	M <sub>11max</sub>	0.096 pl <sup>2</sup>	0.203 F	0.278 F	0.215 F	0.316 F	0.217 F	0.129 K	0.126 K	0.121 K
	M <sub>12max</sub>	-	-	0.222 F	0.145 F	0.383 F	0.318 F	-	-	-
	M <sub>13max</sub>	-	-	-	-	0.200 F	0.085 F	-	-	-
	M <sub>B</sub>	-0.063 pl <sup>2</sup>	-0.094 F	-0.167 F	-0.141 F	-0.234 F	-0.198 F	-0.078 K	-0.078 K	-0.076 K
	R <sub>A</sub> =Q <sub>1Amax</sub>	0.438 pl	0.406 F	0.833 F	0.859 F	1.266 F	1.302 F	0.422 K	0.422 K	0.424 K
	M <sub>11min</sub>	-	-0.047 F	-0.056 F	-0.035 F	-0.059 F	-0.033 F	-0.035 K	-0.035 K	-0.034 K
	M <sub>12min</sub>	-	-	-0.111 F	-0.106 F	-0.117 F	-0.059 F	-	-	-
	M <sub>13min</sub>	-	-	-	-	-0.176 F	-0.165 F	-	-	-
	R <sub>A</sub> =Q <sub>1Amin</sub>	-0.063 pl	-0.094 F	-0.167 F	-0.141 F	-0.234 F	-0.198 F	-0.078 K	0.078 K	-0.076 K

### 7.1 NOSAČI PREKO TRI JEDNAKA RASPONA

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju								
		$x = (0.4 \pm 0.5)L$	$\frac{P}{2}$	$\frac{P}{2}$	$\frac{P}{3} / \frac{1}{1/3}$	$\frac{P}{4} / \frac{1}{1/2} / \frac{P}{4}$	$\frac{P}{4} / \frac{1}{1/4} / \frac{P}{4}$	$\frac{P}{6} / \frac{1}{1/3} / \frac{1}{1/3}$	$\frac{1}{2} / \frac{1}{2}$	$0.4 / 0.2 / 0.4 / 0.4$
0.21051 0.20001	M <sub>11</sub>	0.080 pl <sup>2</sup>	0.175 F	0.244 F	0.194 F	0.281 F	0.197 F	0.108 K	0.107 K	0.102 K
	M <sub>12</sub>	-	-	0.156 F	0.081 F	0.313 F	0.258 F	-	-	-
	M <sub>13</sub>	-	-	-	-	0.094 F	-0.014 F	-	-	-
	M <sub>21</sub>	0.025 pl <sup>2</sup>	0.100 F	0.067 F	0.025 F	0	-0.067 F	0.042 K	0.040 K	0.038 K
	M <sub>22</sub>	-	-	0.067 F	0.025 F	0.125 F	0.100 F	-	-	-
	M <sub>23</sub>	-	-	-	-	0.375 F	-0.317 F	-0.125 K	-0.124 K	-0.121 K
	R <sub>A</sub> =Q <sub>1A</sub>	-0.100 pl <sup>2</sup>	-0.150 F	-0.267 F	-0.225 F	-0.375 F	-0.317 F	-0.125 K	-0.124 K	-0.121 K
	R <sub>B</sub>	0.400 pl	0.350 F	0.733 F	0.775 F	1.125 F	1.183 F	0.375 K	0.376 K	0.379 K
	R <sub>C</sub>	1.100 pl	1.150 F	2.267 F	2.225 F	3.375 F	3.317 F	1.125 K	1.124 K	1.121 K
	Q <sub>1B</sub>	-0.600 pl	-0.650 F	-1.267 F	-1.225 F	-1.875 F	-1.817 F	-0.625 K	-0.624 K	-0.621 K
	Q <sub>2B</sub> =Q <sub>2C</sub>	0.500 pl	0.500 F	1.000 F	1.000 F	1.500 F	1.500 F	0.500 K	0.500 K	0.500 K
	M <sub>11max</sub>	0.101 pl <sup>2</sup>	0.213 F	0.289 F	0.222 F	0.328 F	0.224 F	0.136 K	0.134 K	0.128 K
	M <sub>12max</sub>	-	-	0.244 F	0.166 F	0.406 F	0.338 F	-	-	-
	M <sub>13max</sub>	-	-	-	-	0.234 F	0.118 F	-	-	-
	M <sub>21min</sub>	-0.050 pl <sup>2</sup>	-0.075 F	-0.133 F	-0.113 F	-0.158 F	-0.158 F	-0.063 K	-0.062 K	-0.061 K
	M <sub>22min</sub>	-0.050 pl <sup>2</sup>	-0.075 F	-0.133 F	-0.113 F	-0.188 F	-0.158 F	-0.063 K	-0.062 K	-0.061 K
	M <sub>23min</sub>	-0.450 pl	0.425 F	0.867 F	0.888 F	1.313 F	1.342 F	0.375 K	0.376 K	0.379 K

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
		$x = (0.4 \pm 0.5)l$	$\frac{P}{1/2}$	$\frac{P}{1/2}$	$\frac{P}{1/3} \frac{1}{1/3} \frac{1}{1/3}$	$\frac{P}{1/4} \frac{1}{1/2} \frac{1}{1/4}$	$\frac{P}{1/4} \frac{1}{1/4} \frac{1}{1/4} \frac{1}{1/4}$	$\frac{P}{1/6} \frac{1}{1/3} \frac{1}{1/3} \frac{1}{1/6}$	$\frac{P}{1/2} \frac{1}{1/2}$	$\frac{1/2}{1} \frac{1}{2}$	$0.4 \frac{1}{0.2} 1 \frac{0.4}{1}$
	$M_{11\min}$	$-0.025 \text{ pl}^2$	-0.038 F	-0.044 F	-0.028 F	-0.047 F	-0.026 F	-0.028 K	-0.028 K	-0.027 K	-0.027 K
	$M_{12\min}$	-	-	-0.089 F	-0.084 F	-0.094 F	-0.079 F	-	-	-	-
	$M_{13\min}$	-	-	-	-	-0.141 F	-0.132 F	-	-	-	-
	$M_{21\max}$	$0.075 \text{ pl}^2$	0.175 F	0.200 F	0.138 F	0.188 F (0.100 F)	0.104 K	0.102 K	0.096 K	-	-
	$M_{22\max}$	-	-	0.200 F	0.138 F	0.313 F	0.258 F	-	-	-	-
	$M_B$	$-0.050 \text{ pl}^2$	-0.075 F	-0.133 F	-0.113 F	-0.188 F	-0.158 F	-0.063 K	-0.062 K	-0.061 K	-0.061 K
	$R_A = Q_{1A\min}$	-0.050 pl	-0.075 F	-0.133 F	-0.113 F	-0.188 F	-0.158 F	-0.063 K	-0.062 K	-0.061 K	-0.061 K
	$M_B\min$	$-0.117 \text{ pl}^2$	-0.175 F	-0.311 F	-0.263 F	-0.438 F	-0.369 F	-0.146 K	-0.145 K	-0.142 K	-0.142 K
	$M_C$	$-0.033 \text{ pl}^2$	-0.050 F	-0.089 F	-0.075 F	-0.125 F	-0.106 F	-0.041 K	-0.041 K	-0.041 K	-0.041 K
	$R_B\max$	1.200 pl	1.300 F	2.533 F	2.450 F	3.750 F	3.633 F	1.251 K	1.249 K	1.244 K	1.244 K
	$Q_{1B\min}$	-0.617 F	-1.311 F	-1.283 F	-1.937 F	-1.869 F	-0.648 K	-0.645 K	-0.642 K	-0.642 K	-0.642 K
	$Q_{2B\max}$	0.583 pl	0.625 F	1.222 F	1.188 F	1.813 F	1.764 F	0.605 K	0.604 K	0.602 K	0.602 K
	$M_B\max$	$0.017 \text{ pl}^2$	0.025 F	0.044 F	0.038 F	0.063 F	0.053 F	0.022 K	0.021 K	0.021 K	0.021 K
	$M_C$	$-0.067 \text{ pl}^2$	-0.100 F	-0.178 F	-0.150 F	-0.250 F	-0.211 F	-0.083 K	-0.083 K	-0.081 K	-0.081 K
	$Q_{1B\max}$	0.017 pl	0.025 F	0.044 F	0.038 F	0.063 F	0.053 F	0.022 K	0.021 K	0.021 K	0.021 K
	$Q_{2B\min}$	-0.063 pl	-0.125 F	-0.222 F	-0.188 F	-0.313 F	-0.264 F	-0.105 K	-0.104 K	-0.102 K	-0.102 K

## 7.2 NOSAČI PREKO ČETIRI JEDNAKA RASPONA

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
		$x = (0.4 \pm 0.5)l$	$\frac{P}{1/2} \frac{1}{1/2}$	$\frac{P}{1/3} \frac{1}{1/3} \frac{1}{1/3}$	$\frac{P}{1/4} \frac{1}{1/2} \frac{1}{1/4}$	$\frac{P}{1/4} \frac{1}{1/4} \frac{1}{1/4} \frac{1}{1/4}$	$\frac{P}{1/6} \frac{1}{1/3} \frac{1}{1/3} \frac{1}{1/6}$	$\frac{P}{1/2} \frac{1}{1/2}$	$\frac{1/2}{1} \frac{1}{2}$	$0.4 \frac{1}{0.2} 1 \frac{0.4}{1}$	$0.3 \frac{1}{0.4} 1 \frac{0.3}{1}$
	$M_{11}$	$0.077 \text{ pl}^2$	0.170 F	0.238 F	0.190 F	0.275 F	0.193 F	0.104 K	0.103 K	0.098 K	0.098 K
	$M_{12}$	-	-	0.143 F	0.089 F	0.299 F	0.247 F	-	-	-	-
	$M_{13}$	-	-	-	-	0.074 F	-0.033 F	-	-	-	-
	$M_{21}$	$0.037 \text{ pl}^2$	0.116 F	0.079 F	0.029 F	0.007 F	-0.070 F	0.056 K	0.053 K	0.049 K	0.049 K
	$M_{22}$	-	-	0.111 F	0.069 F	0.165 F	0.134 F	-	-	-	-
	$M_{23}$	-	-	-	-	0.074 F	0.005 F	-	-	-	-
	$M_B$	$-0.107 \text{ pl}^2$	-0.161 F	-0.286 F	-0.241 F	-0.402 F	-0.339 F	-0.134 K	-0.133 K	-0.130 K	-0.130 K
	$M_C$	$-0.071 \text{ pl}^2$	-0.107 F	-0.190 F	-0.161 F	-0.268 F	-0.226 F	-0.089 K	-0.088 K	-0.086 K	-0.086 K
	$R_A = Q_{1A}$	0.393 pl	0.339 F	0.714 F	0.759 F	1.098 F	1.161 F	0.366 K	0.367 K	0.370 K	0.370 K
	$R_B$	1.143 pl	1.214 F	2.381 F	2.321 F	3.536 F	3.452 F	1.179 K	1.178 K	1.174 K	1.174 K
	$R_C$	0.929 pl	0.892 F	1.810 F	1.839 F	2.732 F	2.774 F	0.910 K	0.910 K	0.912 K	0.912 K
	$Q_{1B}$	-0.607 pl	-0.661 F	-1.286 F	-1.241 F	-1.902 F	-1.839 F	-0.634 K	-0.633 K	-0.630 K	-0.630 K
	$Q_{2B}$	0.536 pl	0.554 F	1.095 F	1.080 F	1.634 F	1.613 F	0.545 K	0.545 K	0.544 K	0.544 K
	$Q_{2C}$	-0.464 pl	-0.446 F	-0.905 F	-0.920 F	-1.366 F	-1.387 F	-0.455 K	-0.455 K	-0.456 K	-0.456 K
	$M_{11\max}$	$0.100 \text{ pl}^2$	0.210 F	0.256 F	0.220 F	0.325 F	0.222 F	0.134 K	0.132 K	0.126 K	0.126 K
	$M_{12\max}$	-	-	0.238 F	0.160 F	0.400 F	0.332 F	-	-	-	-
	$M_{13\max}$	-	-	-	-	0.224 F	0.109 F	-	-	-	-
	$M_{21\min}$	$-0.045 \text{ pl}^2$	-0.067 F	-0.127 F	-0.110 F	-0.184 F	-0.160 F*	-0.056 K	-0.056 K	-0.055 K	-0.055 K
	$M_{22\min}$	-	-	-0.111 F	-0.090 F	-0.167 F	-0.141 F	-	-	-	-
	$M_{23\min}$	-	-	-	-	-0.151 F	-0.123 F*	-	-	-	-
	$M_B$	$-0.054 \text{ pl}^2$	-0.080 F	-0.143 F	-0.121 F	-0.201 F	-0.170 F	-0.067 K	-0.067 K	-0.065 K	-0.065 K
	$M_C$	$-0.036 \text{ pl}^2$	-0.054 F	-0.095 F	-0.080 F	-0.134 F	-0.113 F	-0.045 K	-0.045 K	-0.044 K	-0.044 K
	$R_A = Q_{1A\max}$	0.446 pl	0.420 F	0.857 F	0.879 F	1.299 F	1.330 F	0.433 K	0.433 K	0.435 K	0.435 K

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
	$M_{11\min}$	$-0.023 p l^2$	-0.040 Fl	-0.048 Fl	-0.030 Fl	-0.050 Fl	-0.028 Fl	-0.030 Kl	-0.030 Kl	-0.029 Kl	
	$M_{12\min}$	-	-	-0.095 Fl	-0.090 Fl	-0.110 Fl	-0.085 Fl	-	-	-	
	$M_{13\min}$	-	-	-	-	-0.151 Fl	-0.141 Fl	-	-	-	
	$M_{21\max}$	$0.080 p l^2$	0.183 Fl	0.206 Fl	0.140 Fl	0.191 Fl	0.090 Fl* (0.099 Fl)	0.111 Kl	0.108 Kl	0.102 Kl	
	$M_{22\max}$	-	-	0.222 Fl	0.160 Fl	0.333 Fl	0.275 Fl	-	-	-	
	$M_{23\max}$	-	-	-	-	0.224 Fl	0.127 Fl* (0.138 Fl)	-	-	-	
	$M_B$	$-0.054 p l^2$	-0.080 Fl	-0.143 Fl	-0.121 Fl	-0.201 Fl	-0.170 Fl	-0.067 Kl	-0.067 Kl	-0.065 Kl	
	$M_C$	$-0.036 p l^2$	-0.054 Fl	-0.095 Fl	-0.080 Fl	0.134 Fl	-0.113 Fl	-0.045 Kl	-0.045 Kl	-0.044 Kl	
	$R_A = Q_1 A \min$	$-0.054 p l$	-0.080 F	-0.143 F	-0.121 F	-0.201 F	-0.170 F	-0.067 K	-0.067 K	-0.065 K	

\* Označeni način opterećenja ne daje točnu max. odnosno min. vrijednost jer se ova točka nalazi na vanjskom potezu nosača (između stalne točke i ležaja). Granična vrijednost dobivena drugačijim načinom opterećenja dana je u zagradi.

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
	$M_B \min$	$-0.121 p l^2$	-0.181 Fl	-0.321 Fl	-0.271 Fl	-0.452 Fl	-0.382 Fl	-0.151 Kl	-0.150 Kl	-0.146 Kl	
	$M_C$	$-0.018 p l^2$	-0.027 Fl	-0.048 Fl	-0.040 Fl	-0.067 Fl	-0.057 Fl	-0.023 Kl	-0.022 Kl	-0.022 Kl	
	$M_D$	$-0.058 p l^2$	-0.087 Fl	-0.155 Fl	-0.131 Fl	-0.218 Fl	-0.184 Fl	-0.072 Kl	-0.072 Kl	-0.070 Kl	
	$R_B \max$	$1.223 p l$	$1.335 F$	$2.595 F$	$2.502 F$	$3.837 F$	$3.707 F$	$1.279 K$	$1.278 K$	$1.270 K$	
	$Q_{1B} \min$	$-0.621 p l$	-0.681 F	-1.321 F	-1.271 F	-1.952 F	-1.862 F	-0.651 K	-0.650 K	-0.646 K	
	$Q_{2B} \max$	$0.603 p l$	$0.654 F$	$1.274 F$	$1.231 F$	$1.885 F$	$1.825 F$	$0.628 K$	$0.628 K$	$0.624 K$	
	$M_B \max$	$0.013 p l^2$	0.020 Fl	0.036 Fl	0.030 Fl	0.050 Fl	0.042 Fl	0.017 Kl	0.017 Kl	0.016 Kl	
	$M_C$	$-0.054 p l^2$	-0.080 Fl	-0.143 Fl	-0.120 Fl	-0.201 Fl	-0.170 Fl	-0.066 Kl	-0.066 Kl	-0.064 Kl	
	$M_D$	$-0.049 p l^2$	-0.074 Fl	-0.131 Fl	-0.110 Fl	-0.184 Fl	-0.156 Fl	-0.062 Kl	-0.061 Kl	-0.060 Kl	
	$R_B \min$	$-0.080 p l$	-0.121 F	-0.214 F	-0.181 F	-0.301 F	-0.254 F	-0.100 K	-0.100 K	-0.096 K	
	$Q_{1B} \max$	$0.013 p l$	0.020 F	0.036 F	0.030 F	0.050 F	0.042 F	0.017 K	0.017 K	0.016 K	
	$Q_{2B} \min$	$-0.067 p l$	-0.100 F	-0.178 F	-0.151 F	-0.251 F	-0.212 F	-0.083 K	-0.083 K	-0.080 K	
	$M_B$	$-0.036 p l^2$	-0.054 Fl	-0.095 Fl	-0.080 Fl	-0.134 Fl	-0.113 Fl	-0.045 Kl	-0.045 Kl	-0.044 Kl	
	$M_C \min$	$-0.107 p l^2$	-0.161 Fl	-0.286 Fl	-0.241 Fl	-0.402 Fl	-0.339 Fl	-0.134 Kl	-0.133 Kl	-0.130 Kl	
	$R_C \max$	$1.143 p l$	$1.214 F$	$2.381 F$	$2.321 F$	$3.536 F$	$3.452 F$	$1.178 K$	$1.178 K$	$1.172 K$	
	$Q_{2C} \min$	$-0.571 p l$	-0.607 F	-1.191 F	-1.160 F	-1.768 F	-1.726 F	-0.588 K	-0.588 K	-0.586 K	
	$M_B$	$-0.071 p l^2$	-0.107 Fl	-0.190 Fl	-0.161 Fl	-0.268 Fl	-0.226 Fl	-0.089 Kl	-0.088 Kl	-0.086 Kl	
	$M_C \max$	$0.036 p l^2$	0.054 Fl	0.095 Fl	0.080 Fl	0.134 Fl	0.113 Fl	0.045 Kl	0.045 Kl	0.044 Kl	
	$R_C \min$	$-0.214 p l$	-0.321 F	-0.571 F	-0.482 F	-0.804 F	-0.679 F	-0.268 K	-0.266 K	-0.260 K	
	$Q_{2C} \max$	$0.107 p l$	0.161 F	0.286 F	0.241 F	0.402 F	0.339 F	0.134 K	0.133 K	0.130 K	

### 7.3 NOSAČI PREKO PET JEDNAKIH RASPONA

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju								
		$x = (0.4 + 0.5)l$	P 1/2 1/2	P P 1/3 1/3 1/3	P P 1/4 1/4 1/4	P P P 1/4 1/4 1/4 1/4	P P P (6) 1/3 1/3 1/6	1/2 1/2	0.41 0.21 0.41	0.31 0.41 0.31
M <sub>11</sub>	0.078 p <sup>2</sup>	0.171 F	0.240 F	0.191 F	0.276 F	0.194 F	0.106 K	0.104 K	0.099 K	
M <sub>12</sub>	-	-	0.146 F	0.072 F	0.303 F	0.250 F	-	-	-	
M <sub>13</sub>	-	-	-	-	0.079 F	-0.026 F	-	-	-	
M <sub>21</sub>	0.033 p <sup>2</sup>	0.112 F	0.076 F	0.028 F	0.005 F	-0.069 F	0.052 K	0.050 K	0.046 K	
M <sub>22</sub>	-	-	0.099 F	0.058 F	0.155 F	0.125 F	-	-	-	
M <sub>23</sub>	-	-	-	-	0.054 F	-0.014 F	-	-	-	
M <sub>31</sub>	0.046 p <sup>2</sup>	0.132 F	0.123 F	0.072 F	0.079 F	0	0.068 K	0.066 K	0.061 K	
M <sub>32</sub>	-	-	0.123 F	0.072 F	0.204 F	0.167 F	-	-	-	
M <sub>B</sub>	-0.105 p <sup>2</sup>	-0.158 F	-0.281 F	-0.237 F	-0.395 F	-0.333 F	-0.131 K	-0.130 K	-0.127 K	
M <sub>C</sub>	-0.079 p <sup>2</sup>	-0.118 F	-0.211 F	-0.178 F	-0.296 F	-0.250 F	-0.099 K	-0.098 K	-0.096 K	
R <sub>A</sub> =Q <sub>1A</sub>	0.385 pF	0.342 F	0.719 F	0.763 F	1.105 F	1.167 F	0.369 K	0.370 K	0.373 K	
R <sub>B</sub>	1.132 pF	1.197 F	2.351 F	2.296 F	3.494 F	3.417 F	1.163 K	1.162 K	1.158 K	
R <sub>C</sub>	0.974 pF	0.960 F	1.930 F	1.941 F	2.901 F	2.917 F	0.968 K	0.968 K	0.969 K	
Q <sub>1B</sub>	-0.605 pF	-0.658 F	-1.281 F	-1.237 F	-1.895 F	-1.833 F	-0.631 K	-0.630 K	-0.627 K	
Q <sub>2B</sub>	0.526 pF	0.540 F	1.070 F	1.059 F	1.599 F	1.583 F	0.532 K	0.532 K	0.531 K	
Q <sub>2C</sub>	-0.474 pF	-0.460 F	-0.930 F	-0.841 F	-1.401 F	-1.471 F	-0.468 K	-0.468 K	-0.469 K	
Q <sub>3C</sub>	0.500 pF	0.500 F	1.000 F	1.000 F	1.500 F	1.500 F	0.500 K	0.500 K	0.500 K	

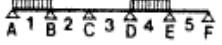
Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju								
		$x = (0.4 + 0.5)l$	P 1/2 1/2	P P 1/3 1/3 1/3	P P 1/4 1/4 1/4	P P P 1/4 1/4 1/4 1/4	P P P (6) 1/3 1/3 1/6	1/2 1/2	0.41 0.21 0.41	0.31 0.41 0.31
M <sub>11</sub> max	0.100 p <sup>2</sup>	0.211 F	0.287 F	0.220 F	0.326 F	0.222 F	0.135 K	0.132 K	0.126 K	
M <sub>12</sub> max	-	-	0.240 F	0.161 F	0.401 F	0.333 F	-	-	-	
M <sub>13</sub> max	-	-	-	-	0.227 F	0.111 F	-	-	-	
M <sub>21</sub> min	-0.046 p <sup>2</sup>	-0.069 F	-0.129 F	-0.111 F	-0.185 F	-0.160 F*	-0.058 K	-0.058 K	-0.056 K	
M <sub>22</sub> min	-	-	-0.117 F	-0.096 F	-0.173 F	-0.146 F	-	-	-	
M <sub>23</sub> min	-	-	-	-	-0.160 F	-0.132 F*	(-0.144 F)	-	-	
M <sub>31</sub> max	0.086 p <sup>2</sup>	0.191 F	0.228 F	0.161 F	0.227 F	-0.125 F*	0.117 K	0.114 K	0.109 K	
M <sub>32</sub> max	-	-	0.228 F	0.161 F	0.352 F	0.292 F	-	-	-	
M <sub>B</sub>	-0.053 pF	-0.079 F	-0.140 F	-0.118 F	-0.197 F	-0.167 F	-0.066 K	-0.066 K	-0.064 K	
M <sub>C</sub>	-0.039 pF	-0.059 F	-0.105 F	-0.089 F	-0.148 F	-0.125 F	-0.050 K	-0.050 K	-0.048 K	
R <sub>A</sub> =Q <sub>1Amax</sub>	0.447 pF	0.421 F	0.860 F	0.882 F	-1.303 F	1.333 F	0.434 K	0.434 K	0.436 K	

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
M <sub>11min</sub>	-0.026 pl <sup>2</sup>	-0.039 Fl	-0.047 Fl	-0.030 Fl	-0.049 Fl	-0.028 Fl	-0.030 Kl	-0.030 Kl	-0.029 Kl		
M <sub>12min</sub>		-	-0.094 Fl	-0.089 Fl	-0.099 Fl	-0.083 Fl	-	-	-		
M <sub>13min</sub>		-	-	-	-0.148 Fl	-0.139 Fl	-	-	-		
M <sub>21max</sub>	0.079 pl <sup>2</sup>	0.181 Fl	0.205 Fl	0.138 Fl	0.190 Fl	0.090 Fl *	0.109 Kl	0.106 Kl	0.101 Kl		
M <sub>22max</sub>		-	0.216 Fl	0.154 Fl	0.327 Fl	0.271 Fl	-	-	-		
M <sub>23max</sub>		-	-	-	0.215 Fl	0.118 Fl *	(0.130 Fl)	-	-		
M <sub>31min</sub>	-0.040 pl <sup>2</sup>	-0.059 Fl	-0.105 Fl	0.089 Fl	-0.148 Fl	-0.125 Fl *	-0.050 Kl	-0.050 Kl	-0.048 Kl		
M <sub>32min</sub>		-	-0.105 Fl	-0.089 Fl	-0.148 Fl	-0.125 Fl	-	-	-		
M <sub>B</sub>	-0.053 pl <sup>2</sup>	-0.079 Fl	-0.140 Fl	-0.118 Fl	-0.197 Fl	-0.167 Fl	-0.066 Kl	-0.066 Kl	-0.064 Kl		
M <sub>C</sub>	-0.039 pl <sup>2</sup>	-0.059 Fl	-0.105 Fl	-0.089 Fl	-0.148 Fl	-0.125 Fl	-0.050 Kl	-0.050 Kl	-0.048 Kl		
R <sub>A</sub> =δ <sub>1Amin</sub>	-0.053 pl	-0.079 F	-0.140 F	-0.118 F	-0.197 F	-0.167 F	-0.066 K	-0.066 K	-0.064 K		

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
M <sub>Bmin</sub>	-0.120 pl <sup>2</sup>	-0.179 Fl	-0.319 Fl	-0.269 Fl	-0.449 Fl	-0.379 Fl	-0.149 Kl	-0.148 Kl	-0.144 Kl		
M <sub>C</sub>	-0.022 pl <sup>2</sup>	-0.032 Fl	-0.057 Fl	-0.048 Fl	-0.081 Fl	-0.068 Fl	-0.027 Kl	-0.027 Kl	-0.027 Kl		
M <sub>D</sub>	-0.044 pl <sup>2</sup>	-0.066 Fl	-0.118 Fl	-0.100 Fl	-0.166 Fl	-0.140 Fl	-0.055 Kl	-0.055 Kl	-0.053 Kl		
M <sub>E</sub>	-0.051 pl <sup>2</sup>	-0.077 Fl	-0.137 Fl	-0.116 Fl	-0.193 Fl	-0.168 Fl	-0.064 Kl	-0.063 Kl	-0.062 Kl		
R <sub>Bmax</sub>	1.218 pl	1.327 F	2.581 F	2.490 F	3.817 F	3.689 F	1.271 K	1.269 K	1.261 K		
Q <sub>1Bmin</sub>	-0.620 pl	-0.679 F	-1.319 F	-1.269 F	-1.949 F	-1.879 F	-0.649 K	-0.648 K	-0.644 K		
Q <sub>2Bmax</sub>	0.598 pl	0.647 F	1.262 F	1.221 F	1.868 F	1.811 F	0.622 K	0.621 K	0.617 K		
M <sub>Bmax</sub>	0.014 pl <sup>2</sup>	0.022 Fl	0.038 Fl	0.032 Fl	0.054 Fl	0.045 Fl	0.018 Kl	0.018 Kl	0.017 Kl		
M <sub>C</sub>	0.057 pl <sup>2</sup>	-0.086 Fl	-0.153 Fl	-0.129 Fl	-0.215 Fl	-0.182 Fl	-0.072 Kl	-0.071 Kl	-0.069 Kl		
M <sub>D</sub>	-0.035 pl <sup>2</sup>	-0.052 Fl	-0.093 Fl	-0.078 Fl	-0.130 Fl	-0.110 Fl	-0.044 Kl	-0.043 Kl	-0.043 Kl		
M <sub>E</sub>	-0.054 pl <sup>2</sup>	-0.081 Fl	-0.144 Fl	-0.121 Fl	-0.202 Fl	-0.170 Fl	-0.067 Kl	-0.067 Kl	-0.065 Kl		
R <sub>Bmin</sub>	-0.086 pl	-0.129 F	-0.230 F	-0.194 F	-0.323 F	-0.273 F	-0.108 K	-0.108 K	-0.103 K		
Q <sub>1Bmax</sub>	0.014 pl	0.022 F	0.038 F	0.032 F	0.054 F	0.045 F	0.018 K	0.018 K	0.017 K		
Q <sub>2Bmin</sub>	-0.072 pl	-0.106 F	-0.191 F	-0.161 F	-0.269 F	-0.227 F	-0.090 K	-0.089 K	-0.086 K		

## TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Način opterećenja	Statička veličina	Način opterećenja u opterećenom polju									
		$x = (0.4 \div 0.5)l$	$P$	$1/2, 1/2$	$P, P, 1/3, 1/3$	$1/4, 1/2, 1/4$	$1/4, 1/4, 1/4, 1/4$	$1/6, 1/3, 1/3, 1/6$	$1/2, 1/2$	$0.41, 0.21, 0.41$	$0.31, 0.41, 0.31$
	$M_B$	$-0.035 pl^2$	$-0.052 Fl$	$-0.093 Fl$	$-0.078 Fl$	$-0.130 Fl$	$-0.110 Fl$	$-0.044 KI$	$-0.043 KI$	$-0.042 KI$	
	$M_C \text{ min}$	$-0.111 pl^2$	$-0.167 Fl$	$-0.297 Fl$	$-0.250 Fl$	$-0.417 Fl$	$-0.352 Fl$	$-0.139 KI$	$-0.138 KI$	$-0.134 KI$	
	$M_D$	$-0.020 pl^2$	$-0.031 Fl$	$-0.054 Fl$	$-0.046 Fl$	$-0.076 Fl$	$-0.064 Fl$	$-0.025 KI$	$-0.025 KI$	$-0.024 KI$	
	$M_E$	$-0.057 pl^2$	$-0.086 Fl$	$-0.153 Fl$	$-0.129 Fl$	$-0.215 Fl$	$-0.182 Fl$	$-0.071 KI$	$-0.071 KI$	$-0.069 KI$	
	$R_{C \text{ max}}$	$1.167 pl$	$1.251 F$	$2.447 F$	$2.377 F$	$3.628 F$	$3.530 F$	$1.209 K$	$1.208 K$	$1.202 K$	
	$Q_{2C \text{ min}}$	$-0.576 pl$	$-0.615 F$	$-1.204 F$	$-1.172 F$	$-1.787 F$	$-1.742 F$	$-0.595 K$	$-0.595 K$	$-0.592 K$	
	$Q_{3C \text{ max}}$	$0.591 pl$	$0.636 F$	$1.242 F$	$1.205 F$	$1.841 F$	$1.788 F$	$0.614 K$	$0.613 K$	$0.610 K$	
	$M_B$	$-0.071 pl^2$	$-0.106 Fl$	$-0.188 Fl$	$-0.159 Fl$	$-0.265 Fl$	$-0.223 Fl$	$-0.087 KI$	$-0.087 KI$	$-0.085 KI$	
	$M_C \text{ max}$	$0.032 pl^2$	$0.048 Fl$	$0.086 Fl$	$0.073 Fl$	$0.121 Fl$	$0.102 Fl$	$0.040 KI$	$0.040 KI$	$0.038 KI$	
	$M_D$	$-0.059 pl^2$	$-0.088 Fl$	$-0.156 Fl$	$-0.132 Fl$	$-0.220 Fl$	$-0.186 Fl$	$-0.074 KI$	$-0.073 KI$	$-0.072 KI$	
	$M_E$	$-0.048 pl^2$	$-0.072 Fl$	$-0.128 Fl$	$-0.108 Fl$	$-0.179 Fl$	$-0.152 Fl$	$-0.060 KI$	$-0.059 KI$	$-0.058 KI$	
	$R_{C \text{ min}}$	$-0.194 pl$	$-0.291 F$	$-0.517 F$	$-0.436 F$	$-0.727 F$	$-0.614 F$	$-0.241 K$	$-0.240 K$	$-0.233 K$	
	$Q_{2C \text{ max}}$	$0.103 pl$	$0.154 F$	$0.274 F$	$0.232 F$	$0.386 F$	$0.326 F$	$0.127 K$	$0.127 K$	$0.123 K$	
	$Q_{3C \text{ min}}$	$-0.091 pl$	$-0.136 F$	$-0.242 F$	$-0.205 F$	$-0.341 F$	$-0.288 F$	$-0.114 K$	$-0.113 K$	$-0.110 K$	

## 7.4 NOSAČI PREKO PET I VIŠE JEDNAKIH RASPONA

Oznake	Sva polja potpuno opterećena					Minimum					Maksimum					
	Broj polja					Broj polja					Broj polja					
	5	6	7	8	$\infty$	5	6	7	8	$\infty$	5	6	7	8	$\infty$	
Moment na ležaju	$\alpha$					$\gamma$					$\beta$					
M <sub>1</sub>	-0.1053	-0.1058	-0.1056	-0.1057	-0.1057	-0.1196	-0.1199	-0.1198	-0.1198	-0.1198	+0.0143	+0.0141	+0.0142	+0.0142	+0.0141	
M <sub>2</sub>	-0.0789	-0.0769	-0.0775	-0.0773	-0.0774	-0.1112	-0.1102	-0.1106	-0.1105	-0.1105	+0.0323	+0.0333	+0.0331	+0.0331	+0.0331	
M <sub>3</sub>		-0.0865	-0.0845	-0.0851	-0.0848		-0.1154	-0.1144	-0.1147	-0.1147		+0.0289	+0.0299	+0.0295	+0.0299	
M <sub>4</sub>			-0.0825	-0.0830	-0.0833			-0.1134	-0.1136	-0.1139			+0.0309	+0.0306	+0.0304	
M <sub>5</sub>				-0.0835	-0.0833				-0.1138	-0.1138				+0.0305	+0.0304	
M <sub>6</sub>															+0.0305	
Moment u polju	M <sub>p1</sub>	+0.0779	+0.0777	+0.0778	+0.0777	+0.0777	-0.0222	-0.0223	-0.0221	-0.0222	-0.0222	+0.1002	+0.0999	+0.0999	+0.0999	+0.0999
	M <sub>p2</sub>	+0.0332	+0.0341	+0.0338	+0.0339	+0.0339	-0.0458	-0.0454	-0.0455	-0.0454	-0.0454	+0.0790	+0.0795	+0.0793	+0.0794	+0.0793
	M <sub>p3</sub>	+0.0461	+0.0433	+0.0440	+0.0438	+0.0439	-0.0394	-0.0408	-0.0405	-0.0406	-0.0405	+0.0855	+0.0841	+0.0845	+0.0844	+0.0844
	M <sub>p4</sub>			+0.0405	+0.0412	+0.0411		-0.0423	-0.0432	-0.0419	-0.0418	-0.0417	+0.0828	+0.0844	+0.0830	+0.0835
	M <sub>p5</sub>				+0.0417										+0.0833	
	M <sub>p6</sub>					+0.0416										
Udaljenost maksimalnog momenta od lijevog ležaja	X <sub>1</sub>	0.395	0.394	0.394	0.394	0.394						0.441	0.447	0.447	0.447	0.447
	X <sub>2</sub>	0.526	0.533	0.528	0.528	0.528						0.513	0.514	0.514	0.514	0.514
	X <sub>3</sub>	0.500	0.490	0.493	0.492	0.492						0.500	0.495	0.496	0.496	0.496
	X <sub>4</sub>			0.500	0.503	0.502							0.500	0.501	0.499	0.499
	X <sub>5</sub>					0.499										
	X <sub>6</sub>					0.500										

Nosač preko pet i više jednakih raspona opterećen kontinuiranim opterećenjem

Oznake	Sva polja potpuno opterećena					Minimum					Maksimum					
	Broj polja					Broj polja					Broj polja					
	5	6	7	8	$\infty$	5	6	7	8	$\infty$	5	6	7	8	$\infty$	
	$a$					$c$					$b$					
Poprečna sila [levo desno od ležaja]	Q <sub>0</sub>	+0.3947	+0.3942	+0.3944	+0.3943	+0.3943	-0.0526	-0.0529	-0.0528	-0.0528	-0.0528	+0.4473	+0.4471	+0.4472	+0.4471	+0.4471
	Q <sub>1</sub>	-0.6052	-0.6056	-0.6056	-0.6056	-0.6057	-0.5196	-0.6199	-0.6198	-0.6198	-0.6198	+0.0144	+0.0141	+0.0142	+0.0142	+0.0142
	Q <sub>2</sub>	+0.5263	+0.5288	+0.5282	+0.5284	+0.5284	-0.0718	-0.0705	-0.0709	-0.0708	-0.0707	+0.5981	+0.5993	+0.5991	+0.5991	+0.5991
	Q <sub>3</sub>	-0.4737	-0.4712	-0.4718	-0.4717	-0.4717	-0.3765	-0.5753	-0.5757	-0.5756	-0.5755	+0.1029	+0.1042	+0.1038	+0.1039	+0.1038
	Q <sub>4</sub>	+0.5000	+0.4903	+0.4930	+0.4922	+0.4924	-0.0909	-0.0659	-0.0946	-0.0949	-0.0947	+0.5909	+0.5862	+0.5875	+0.5871	+0.5871
	Q <sub>5</sub>		-0.5096	-0.5070	-0.5078	-0.5076		-0.5962	-0.5949	-0.5953	-0.5951		+0.0865	+0.0879	+0.0875	+0.0875
	Q <sub>6</sub>			+0.5000	+0.5025	+0.5020			-0.0915	-0.0902	-0.0903			+0.5915	+0.5927	+0.5923
	Q <sub>7</sub>				-0.4974	-0.4980				-0.5902	-0.5904				+0.0928	+0.0924
	Q <sub>8</sub>					+0.4994					-0.0917				+0.5911	
	Q <sub>9</sub>					-0.5006					-0.5918				+0.0912	
	Q <sub>10</sub>					+0.5001					-0.0914				+0.5914	
	Q <sub>11</sub>					-0.5000					-0.5914				+0.0914	
Ledične reakcije	R <sub>0</sub>	+0.3947	+0.3942	+0.3944	+0.3943	+0.3943	-0.0526	-0.0529	-0.0528	-0.0528	-0.0528	+0.4473	+0.4471	+0.4472	+0.4471	+0.4471
	R <sub>1</sub>	+1.1316	+1.1346	+1.1338	+1.1340	+1.1341	-0.0861	-0.0847	-0.0860	-0.0849	-0.0849	+1.2177	+1.2155	+1.2188	+1.2189	+1.2190
	R <sub>2</sub>	+0.9737	+0.9615	+0.9649	+0.9639	+0.9641	-0.1938	-0.2000	-0.1983	-0.1989	-0.1985	+1.1675	+1.1615	+1.1622	+1.1626	+1.1626
	R <sub>3</sub>		+1.0182	+1.0070	+1.0103	+1.0065		-0.1731	-0.1794	-0.1778	-0.1779		+1.1923	+1.1864	+1.1881	+1.1874
	R <sub>4</sub>				+0.9948	+0.9974				-0.1856	-0.1841			+1.1804	+1.1815	
	R <sub>5</sub>					+1.0007				-0.1826				+1.1833		
	R <sub>6</sub>					+0.9999				-0.1830				+1.1829		

Minimalni ležajni moment  $M_{n1} = (ag + \beta p)^2$

Maksimalni moment u polju  $M_{np} = (ng + \beta p)^2$

Udaljenost maksimalnog momenta od lijevog ležaja

$x_n = \frac{p}{g}$  I - za totalno opterećenje

$x_n = v$  I - za opterećenje u najnepovoljnijem položaju za  $M_{np}$

Maksimalna poprečna sila  $Q_{n1} = (a - g + b - p) I$  (a s predznakom plus)

Minimalna poprečna sila  $Q_{n2} = (a - g + c - p) I$  (a s predznakom minus)

Maksimalna reakcija  $R_{n1} = (r - g + s - p) I$

Minimalna reakcija  $R_{n2} = (r - g + t - p) I$

**TABLICE 8. PRAKTIČNO DIMENZIONIRANJE PRAVOKUTNOG PRESJEKA**

$\varepsilon_{c2}$ (%)	$\varepsilon_{s1}$ (%)	$\xi = x/d$	$\zeta = z/d$	$\alpha_1$	$\mu_{Sd}$	$k_d$
-0.1	20.0	0.005	0.998	0.000	0.000	$\infty$
-0.2	20.0	0.010	0.997	0.001	0.001	31.623
-0.3	20.0	0.015	0.995	0.002	0.002	22.361
-0.4	20.0	0.020	0.993	0.003	0.003	18.257
-0.5	20.0	0.024	0.992	0.005	0.005	14.142
-0.6	20.0	0.029	0.990	0.007	0.007	11.952
-0.7	20.0	0.034	0.988	0.009	0.009	10.541
-0.8	20.0	0.038	0.987	0.011	0.011	9.535
-0.9	20.0	0.043	0.985	0.014	0.014	8.452
-1.0	20.0	0.048	0.983	0.017	0.017	7.670
-1.1	20.0	0.052	0.982	0.020	0.019	7.255
-1.2	20.0	0.057	0.980	0.023	0.023	6.594
-1.3	20.0	0.061	0.978	0.026	0.026	6.202
-1.4	20.0	0.065	0.977	0.030	0.029	5.872
-1.5	20.0	0.070	0.975	0.033	0.033	5.505
-1.6	20.0	0.074	0.973	0.037	0.036	5.270
-1.7	20.0	0.078	0.971	0.040	0.039	5.064
-1.8	20.0	0.083	0.969	0.044	0.043	4.822
-1.9	20.0	0.087	0.968	0.048	0.046	4.663
-2.0	20.0	0.091	0.966	0.052	0.050	4.472
-2.1	20.0	0.095	0.964	0.055	0.053	4.344
-2.2	20.0	0.099	0.962	0.059	0.056	4.226
-2.3	20.0	0.103	0.960	0.062	0.060	4.082
-2.4	20.0	0.107	0.959	0.066	0.063	3.984
-2.5	20.0	0.111	0.957	0.069	0.066	3.892
-2.6	20.0	0.115	0.955	0.073	0.069	3.807
-2.7	20.0	0.119	0.953	0.076	0.073	3.701
-2.8	20.0	0.123	0.951	0.080	0.076	3.627
-2.9	20.0	0.127	0.949	0.083	0.079	3.558
-3.0	20.0	0.130	0.947	0.086	0.081	3.514
-3.1	20.0	0.134	0.945	0.089	0.084	3.450
-3.2	20.0	0.138	0.943	0.093	0.088	3.371
-3.3	20.0	0.142	0.942	0.096	0.091	3.315
-3.4	20.0	0.145	0.940	0.099	0.093	3.279
-3.5	20.0	0.149	0.938	0.103	0.096	3.227
-3.5	19.5	0.152	0.937	0.105	0.098	3.194
-3.5	19.0	0.156	0.935	0.107	0.100	3.162
-3.5	18.5	0.159	0.934	0.109	0.102	3.131

## TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

$\varepsilon_{c2}$ (%)	$\varepsilon_{s1}$ (%)	$\xi = x/d$	$\zeta = z/d$	$\omega_1$	$\mu_{Sd}$	$k_d$
-3.5	18.0	0.163	0.932	0.112	0.105	3.086
-3.5	17.5	0.167	0.931	0.115	0.107	3.057
-3.5	17.0	0.171	0.929	0.118	0.109	3.029
-3.5	16.5	0.175	0.927	0.120	0.112	2.988
-3.5	16.0	0.179	0.926	0.123	0.114	2.962
-3.5	15.5	0.184	0.923	0.127	0.117	2.924
-3.5	15.0	0.189	0.921	0.130	0.120	2.887
-3.5	14.5	0.194	0.919	0.133	0.123	2.851
-3.5	14.0	0.200	0.917	0.138	0.126	2.817
-3.5	13.5	0.206	0.914	0.142	0.130	2.774
-3.5	13.0	0.212	0.912	0.146	0.133	2.742
-3.5	12.5	0.219	0.909	0.151	0.137	2.702
-3.5	12.0	0.226	0.906	0.156	0.141	2.663
-3.5	11.5	0.233	0.903	0.160	0.145	2.626
-3.5	11.0	0.241	0.900	0.166	0.149	2.591
-3.5	10.5	0.250	0.896	0.172	0.154	2.548
-3.5	10.0	0.259	0.892	0.178	0.159	2.508
-3.5	9.5	0.269	0.888	0.185	0.164	2.469
-3.5	9.0	0.280	0.884	0.193	0.170	2.425
-3.5	8.5	0.292	0.879	0.201	0.177	2.377
-3.5	8.0	0.304	0.874	0.209	0.183	2.338
-3.5	7.5	0.318	0.868	0.219	0.190	2.294
-3.5	7.0	0.333	0.861	0.229	0.197	2.253
-3.5	6.5	0.350	0.854	0.241	0.206	2.203
-3.5	6.0	0.368	0.847	0.253	0.214	2.162
-3.5	5.5	0.389	0.838	0.268	0.224	2.113
-3.5	5.0	0.412	0.829	0.283	0.235	2.063
-3.5	4.5	0.438	0.818	0.301	0.247	2.012
-3.5	4.0	0.467	0.806	0.321	0.259	1.965
-3.5	3.5	0.500	0.792	0.344	0.272	1.917
-3.5	3.0	0.538	0.776	0.370	0.287	1.867
-3.5	2.5	0.583	0.757	0.401	0.304	1.814
-3.5	2.0	0.636	0.735	0.438	0.322	1.762
-3.5	1.5	0.700	0.709	0.482	0.342	1.710
-3.5	1.0	0.778	0.676	0.535	0.362	1.662
-3.5	0.5	0.875	0.636	0.602	0.383	1.616
-0.1	10.0	0.010	0.997	0.000	0.000	$\infty$
-0.2	10.0	0.020	0.993	0.002	0.002	22.361
-0.3	10.0	0.029	0.990	0.004	0.003	18.257
-0.4	10.0	0.038	0.987	0.006	0.006	12.910

## TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

$\varepsilon_{c2}$ (%)	$\varepsilon_{s1}$ (%)	$\xi = x/d$	$\zeta = z/d$	$\omega_1$	$\mu_{Sd}$	$k_d$
-0.5	10.0	0.048	0.984	0.009	0.009	10.541
-0.6	10.0	0.057	0.980	0.013	0.013	8.771
-0.7	10.0	0.065	0.978	0.017	0.017	7.670
-0.8	10.0	0.074	0.974	0.022	0.021	6.901
-0.9	10.0	0.083	0.971	0.027	0.026	6.202
-1.0	10.0	0.091	0.968	0.032	0.031	5.680
-1.1	10.0	0.099	0.965	0.038	0.036	5.270
-1.2	10.0	0.107	0.962	0.044	0.042	4.880
-1.3	10.0	0.115	0.959	0.050	0.048	4.564
-1.4	10.0	0.123	0.956	0.056	0.054	4.303
-1.5	10.0	0.130	0.953	0.062	0.059	4.117
-1.6	10.0	0.138	0.950	0.069	0.065	3.922
-1.7	10.0	0.145	0.947	0.075	0.071	3.753
-1.8	10.0	0.153	0.944	0.082	0.077	3.604
-1.9	10.0	0.160	0.940	0.088	0.083	3.471
-2.0	10.0	0.167	0.937	0.095	0.089	3.352
-2.1	10.0	0.174	0.934	0.101	0.094	3.262
-2.2	10.0	0.180	0.931	0.107	0.099	3.178
-2.3	10.0	0.187	0.928	0.113	0.105	3.086
-2.4	10.0	0.194	0.925	0.119	0.110	3.015
-2.5	10.0	0.200	0.922	0.125	0.115	2.949
-2.6	10.0	0.206	0.919	0.130	0.120	2.887
-2.7	10.0	0.213	0.915	0.136	0.125	2.828
-2.8	10.0	0.219	0.912	0.142	0.129	2.784
-2.9	10.0	0.225	0.910	0.147	0.134	2.732
-3.0	10.0	0.231	0.906	0.153	0.138	2.692
-3.1	10.0	0.237	0.903	0.158	0.143	2.644
-3.2	10.0	0.242	0.901	0.163	0.147	2.608
-3.3	10.0	0.248	0.898	0.168	0.151	2.573
-3.4	10.0	0.254	0.895	0.174	0.155	2.540
-3.5	10.0	0.259	0.892	0.178	0.159	2.508
-0.1	5.0	0.020	0.993	0.001	0.001	31.623
-0.2	5.0	0.038	0.987	0.003	0.003	18.257
-0.3	5.0	0.057	0.981	0.007	0.007	11.952
-0.4	5.0	0.074	0.975	0.012	0.011	9.535
-0.5	5.0	0.091	0.969	0.018	0.017	7.670
-0.6	5.0	0.107	0.963	0.025	0.024	6.455
-0.7	5.0	0.123	0.958	0.032	0.031	5.680
-0.8	5.0	0.138	0.952	0.041	0.039	5.064
-0.9	5.0	0.153	0.947	0.050	0.047	4.613

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

$\varepsilon_{c2}$ (%)	$\varepsilon_{s1}$ (%)	$\xi = x/d$	$\zeta = z/d$	$\omega_1$	$\mu_{sd}$	$k_d$
-1.0	5.0	0.167	0.942	0.059	0.056	4.226
-1.1	5.0	0.180	0.937	0.069	0.064	3.953
-1.2	5.0	0.194	0.931	0.079	0.074	3.676
-1.3	5.0	0.206	0.927	0.089	0.083	3.471
-1.4	5.0	0.219	0.921	0.100	0.092	3.297
-1.5	5.0	0.231	0.917	0.110	0.101	3.147
-1.6	5.0	0.242	0.912	0.121	0.110	3.015
-1.7	5.0	0.254	0.907	0.132	0.119	2.899
-1.8	5.0	0.265	0.902	0.142	0.128	2.795
-1.9	5.0	0.275	0.898	0.152	0.136	2.712
-2.0	5.0	0.286	0.893	0.162	0.145	2.626
-2.1	5.0	0.296	0.888	0.172	0.153	2.557
-2.2	5.0	0.306	0.883	0.181	0.160	2.500
-2.3	5.0	0.315	0.879	0.190	0.167	2.447
-2.4	5.0	0.324	0.874	0.199	0.174	2.397
-2.5	5.0	0.333	0.870	0.208	0.181	2.351
-2.6	5.0	0.342	0.865	0.216	0.187	2.312
-2.7	5.0	0.351	0.861	0.225	0.193	2.276
-2.8	5.0	0.359	0.857	0.232	0.199	2.242
-2.9	5.0	0.367	0.852	0.240	0.205	2.209
-3.0	5.0	0.375	0.848	0.248	0.210	2.182
-3.1	5.0	0.383	0.844	0.256	0.216	2.152
-3.2	5.0	0.390	0.840	0.262	0.221	2.127
-3.3	5.0	0.398	0.836	0.270	0.226	2.104
-3.4	5.0	0.405	0.832	0.277	0.230	2.085
-3.5	5.0	0.412	0.829	0.283	0.235	2.063

Limitirajuće vrijednosti za jednostruko armiranje - dostatna duktilnost

GA 240/360

RA 400/500

B 500/550

$$\varepsilon_{s1,lim} = 1.04 \text{ (%)}$$

$$\varepsilon_{s1,lim} = 1.74 \text{ (%)}$$

$$\varepsilon_{s1,lim} = 2.17 \text{ (%)}$$

$$\xi_{lim} = 0.771$$

$$\xi_{lim} = 0.668$$

$$\xi_{lim} = 0.617$$

$$\alpha_v = 0.8095$$

$$\zeta_{lim} = 0.679$$

$$\zeta_{lim} = 0.722$$

$$\zeta_{lim} = 0.744$$

$$\mu_{Rd,lim} = 0.360$$

$$\mu_{Rd,lim} = 0.332$$

$$\mu_{Rd,lim} = 0.316$$

$$k_a = 0.4160$$

$$k_{d,lim} = 1.667$$

$$k_{d,lim} = 1.736$$

$$k_{d,lim} = 1.779$$

Limitirajuće vrijednosti za jednostruko armiranje - izvanredna duktilnost

C 12/15 + C 35/45

C 40/50 i više

U teoriji plastičnosti

$$\xi_{lim} = 0.45$$

$$\xi_{lim} = 0.35$$

$$\xi_{lim} = 0.25$$

$$\mu_{Rd,lim} = 0.252$$

$$\mu_{Rd,lim} = 0.206$$

$$\mu_{Rd,lim} = 0.154$$

$$k_{d,lim} = 1.993$$

$$k_{d,lim} = 2.204$$

$$k_{d,lim} = 2.547$$

**TABLICA 9. TABLICA REBRASTE ARMATURE**RA-400/500-1  
Ø 6, 8, 10, 12 i 14

RA-400/500-2

Ø 6, 8, 10, 12, 14, 16, 19, 22, 25, 28, 32, 36 i 40

∅ mm	Površina presjeka u cm <sup>2</sup> za komada															Masa kg/m	Opseg cm	Promjer mm
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15			
6	0.28	0.57	0.85	1.13	1.41	1.70	1.98	2.26	2.55	2.83	3.11	3.39	3.68	3.96	4.24	0.230	0.228	1.89
8	0.50	1.01	1.51	2.01	2.51	3.02	3.52	4.02	4.52	5.03	5.53	6.03	6.54	7.04	7.54	0.409	0.405	2.51
10	0.79	1.57	2.36	3.14	3.93	4.71	5.50	6.20	7.07	7.85	8.64	9.42	10.21	11.00	11.78	0.649	0.633	3.14
12	1.13	2.26	3.39	4.52	5.66	6.79	7.92	9.05	10.18	11.31	12.44	13.57	14.70	15.83	16.97	0.920	0.911	3.77
14	1.54	3.08	4.62	6.16	7.70	9.24	10.78	12.32	13.86	15.39	16.93	18.47	20.01	21.55	23.09	1.252	1.242	4.40
16	2.01	4.02	6.03	8.04	10.05	12.06	14.07	16.09	18.10	20.11	22.12	24.13	26.14	28.15	30.16	1.621	1.609	5.09
19	2.84	5.67	8.51	11.34	14.18	17.01	19.85	22.68	25.52	28.35	31.19	34.02	36.86	39.69	42.53	2.288	2.288	5.97
22	3.80	7.60	11.40	15.21	19.01	22.81	26.54	30.41	34.21	38.01	41.81	45.62	49.42	53.22	57.02	3.058	3.058	6.91
25	4.91	9.82	14.73	19.64	24.54	29.45	34.36	39.27	44.18	49.09	54.00	58.90	63.81	68.72	73.63	3.951	3.951	7.85
28	6.16	12.32	18.47	24.63	30.79	36.95	43.10	49.26	55.42	61.58	67.73	73.88	80.05	86.21	92.36	4.956	4.956	8.80
32	8.04	16.09	24.13	32.17	40.21	48.26	56.30	64.34	72.38	80.42	88.47	96.50	104.55	112.60	120.64	6.474	6.474	10.05
36	10.18	20.36	30.54	40.72	50.89	61.07	71.25	81.43	91.61	101.79	111.97	122.15	132.32	142.50	152.68	8.200	8.200	11.31
40	12.57	25.13	37.70	50.27	62.83	75.40	87.97	100.53	113.10	125.66	138.23	150.80	163.36	175.93	188.50	10.117	10.117	12.57
	100	50	33.34	25	20	16.67	14.30	12.50	11.11	10	9.10	8.33	7.70	7.14	6.67	Odgovarajući razmak u cm za 1m širine ploče		

**TABLICA 10. TABLICA ZA ODABIR ARMATURE (PLOČA, ZID)**

Razmak šipaka	Površina presjeka u $\text{cm}^2$ na širini ploče od 100 cm									
	Promjer šipke u mm									
	1	6	7	8	10	12	14	16	18	20
7.0	4.04	5.50	7.18	11.22	16.16	21.99	28.73	36.36	44.87	
7.5	3.77	5.13	6.70	10.47	15.08	20.52	26.81	33.93	41.88	
8.0	3.53	4.81	6.28	9.82	14.14	19.24	25.14	31.81	39.26	
8.5	3.33	4.53	5.91	9.24	13.31	18.11	23.66	29.94	36.95	
9.0	3.14	4.28	5.59	8.73	12.57	17.10	22.34	28.28	34.90	
9.5	2.98	4.05	5.29	8.27	11.90	16.20	21.17	26.79	33.06	
10.0	2.83	3.85	5.03	7.85	11.31	15.39	20.11	25.45	31.41	
10.5	2.69	3.67	4.79	7.48	10.77	14.66	19.15	24.24	29.91	
11.0	2.57	3.50	4.57	7.14	10.28	13.99	18.28	23.14	28.55	
11.5	2.46	3.35	4.37	6.83	9.84	13.39	17.49	22.13	27.31	
12.0	2.36	3.21	4.19	6.54	9.42	12.83	16.76	21.21	26.17	
12.5	2.26	3.08	4.02	6.28	9.05	12.32	16.09	20.36	25.13	
13.0	2.17	2.96	3.87	6.04	8.70	11.84	15.47	19.58	24.16	
13.5	2.09	2.85	3.72	5.82	8.38	11.40	14.90	18.85	23.27	
14.0	2.02	2.75	3.59	5.61	8.08	11.00	14.36	18.18	22.44	
14.5	1.95	2.65	3.47	5.42	7.80	10.62	13.87	17.55	21.66	
15.0	1.89	2.57	3.35	5.24	7.54	10.26	13.41	16.97	20.94	
15.5	1.82	2.48	3.24	5.07	7.30	9.93	12.97	16.42	20.27	
16.0	1.77	2.41	3.14	4.91	7.07	9.62	12.57	15.90	19.64	
16.5	1.71	2.33	3.05	4.76	6.85	9.33	12.19	15.42	19.04	
17.0	1.66	2.26	2.96	4.62	6.65	9.05	11.83	14.97	18.48	
17.5	1.62	2.20	2.87	4.49	6.46	8.79	11.49	14.54	17.95	
18.0	1.57	2.14	2.79	4.36	6.28	8.55	11.17	14.14	17.46	
18.5	1.53	2.08	2.72	4.25	6.11	8.32	10.87	13.76	16.94	
19.0	1.49	2.03	2.65	4.13	5.95	8.10	10.58	13.39	16.54	
19.5	1.45	1.97	2.58	4.03	5.80	7.89	10.31	13.05	16.11	
20.0	1.41	1.92	2.51	3.93	5.65	7.69	10.05	12.72	15.72	

**TABLICA 11. TABLICA MREŽASTE ARMATURE**

Standardne mreže uzdužno nosive (tip R)

Oznaka mreže	Uzdužno nosive mreže (R mreže)						Duljina mreže (mm)	Masa kg/m <sup>2</sup>	Masa kg/kom (tolerancija 1%)			
	Promjer žica (mm)		Razmak žica (mm)		Površina presjeka žica (cm <sup>2</sup> /m)							
	uzdužne D	poprečne d	uzdužno a	poprečno t	uzdužnih	poprečnih						
R-131	5.0	4.0	150	250	1.31	0.50	5000	1.52	16.34			
R-139	4.2	4.2	100	250	1.39	0.55	5000	1.53	16.83			
R-166	4.6	4.2	100	250	1.66	0.55	5000	1.74	19.14			
R-196	5.0	4.2	100	250	1.96	0.55	5000	1.98	21.89			
R-221	6.5	4.6	150	250	2.21	0.66	6000	2.27	30.57			
R-238	5.5	4.2	100	250	2.38	0.55	6000	2.31	30.61			
R-283	6.0	4.2	100	250	2.83	0.55	6000	2.66	35.23			
R-335	8.0	5.0	150	250	3.35	0.78	6000	3.41	43.99			
R-378	8.5	5.0	150	250	3.78	0.78	6000	3.77	48.63			
R-385	7.0	5.0	100	250	3.85	0.78	6000	3.64	48.30			
R-424	9.0	6.0	150	250	4.24	1.13	6000	4.22	57.02			
R-503	8.0	6.0	100	250	5.03	1.13	6000	4.84	64.21			
R-524	10.0	6.0	150	250	5.24	1.13	6000	5.24	67.60			
R-636	9.0	6.0	100	250	6.36	1.13	6000	5.88	78.07			
R-785	10.0	6.0	100	250	7.85	1.13	6000	7.06	93.78			
R-1130	12.0	8.0	100	250	11.30	2.01	6000	10.46	138.98			

l - duljina armaturne mreže

b - širina armaturne mreže

x<sub>1</sub>, x<sub>2</sub> - prepusti uzdužnih žica preko krajnjih poprečnih žicay<sub>1</sub>, y<sub>2</sub> - prepusti poprečnih žica preko krajnjih uzdužnih žica

$$10 \text{ mm} \leq x_1, x_2 \leq \frac{t}{2}$$

$$10 \text{ mm} \leq y_1, y_2 \leq \frac{a}{2}$$

Širina za sve R mreže je 2150 mm.

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Standardne mreže obostrano nosive (tip Q)

Oznaka mreže	Promjer žice (mm)		Razmak žica (mm)		Površina presjeka žica (cm <sup>2</sup> /m)		Duljina mreže (mm)	Masa kg/m <sup>2</sup>	Masa kg/kom (sa 1% tolerancije)
	uzdužne D	poprečne d	uzdužno a	poprečno t	uzdužnih	poprečnih			
Q-131	5.0	5.0	150	150	1.31	1.31	5100	2.12	23.25
Q-188	6.0	6.0	150	150	1.88	1.88	5100	3.05	33.44
Q-221	6.5	6.5	150	150	2.21	2.21	5100	3.60	39.47
Q-257	7.0	7.0	150	150	2.57	2.57	5100	4.16	45.61
Q-283	6.0	6.0	100	100	2.83	2.83	5100	4.44	49.68
Q-335	8.0	8.0	150	150	3.35	3.35	5100	5.44	59.65
Q-503	8.0	8.0	100	100	5.03	5.03	5100	7.90	88.49
Q-785	10.0	10.0	100	100	7.85	7.85	5100	12.34	138.18
Q-1130	12.0	12.0	100	100	11.30	11.30	5100	17.76	198.95

$$10 \text{ mm} \leq x_1, x_2 \leq \frac{t}{2}$$

$$10 \text{ mm} \leq y_1, y_2 \leq \frac{a}{2}$$

Širina za sve mreže 2150 mm.

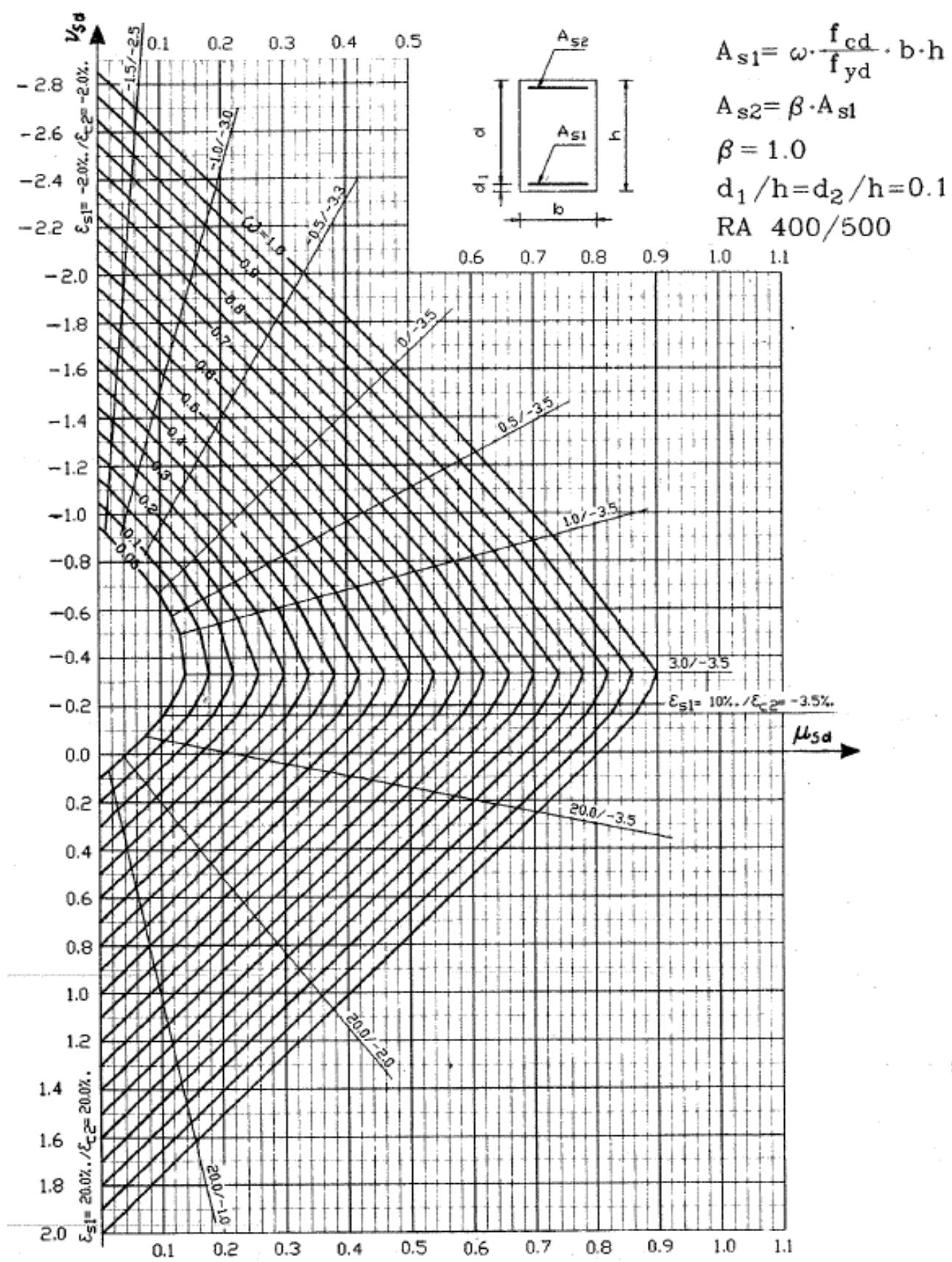
## TABLICA 12. NAJVJEĆI DOPUŠTENI BROJ KOMADA ARMAUTRE U JEDNOJ RAZINI ŠIRINE GREDE

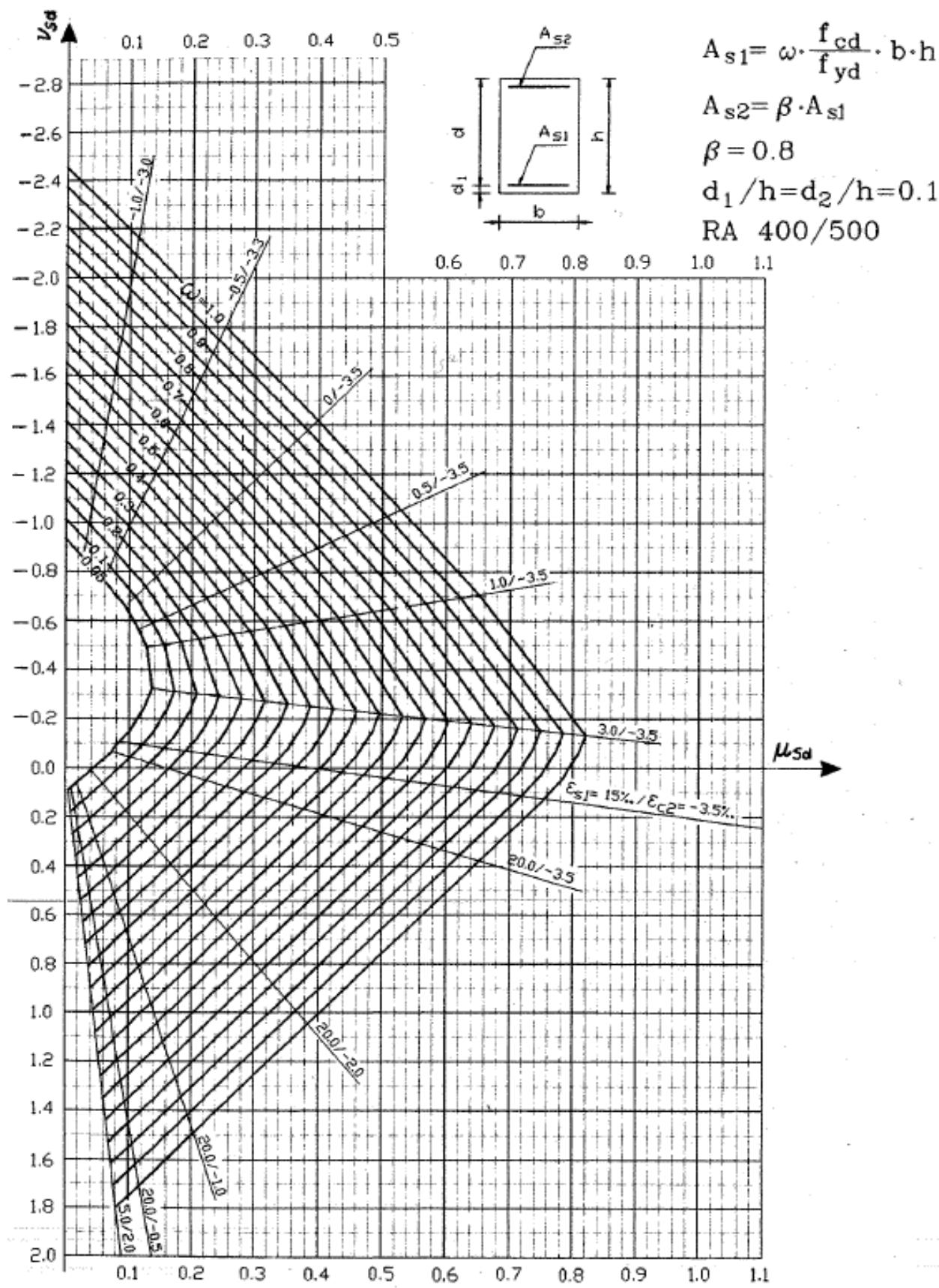
Tabelica 5.10 Najveći dopušteni broj komada armature u jednoj razini širine grede

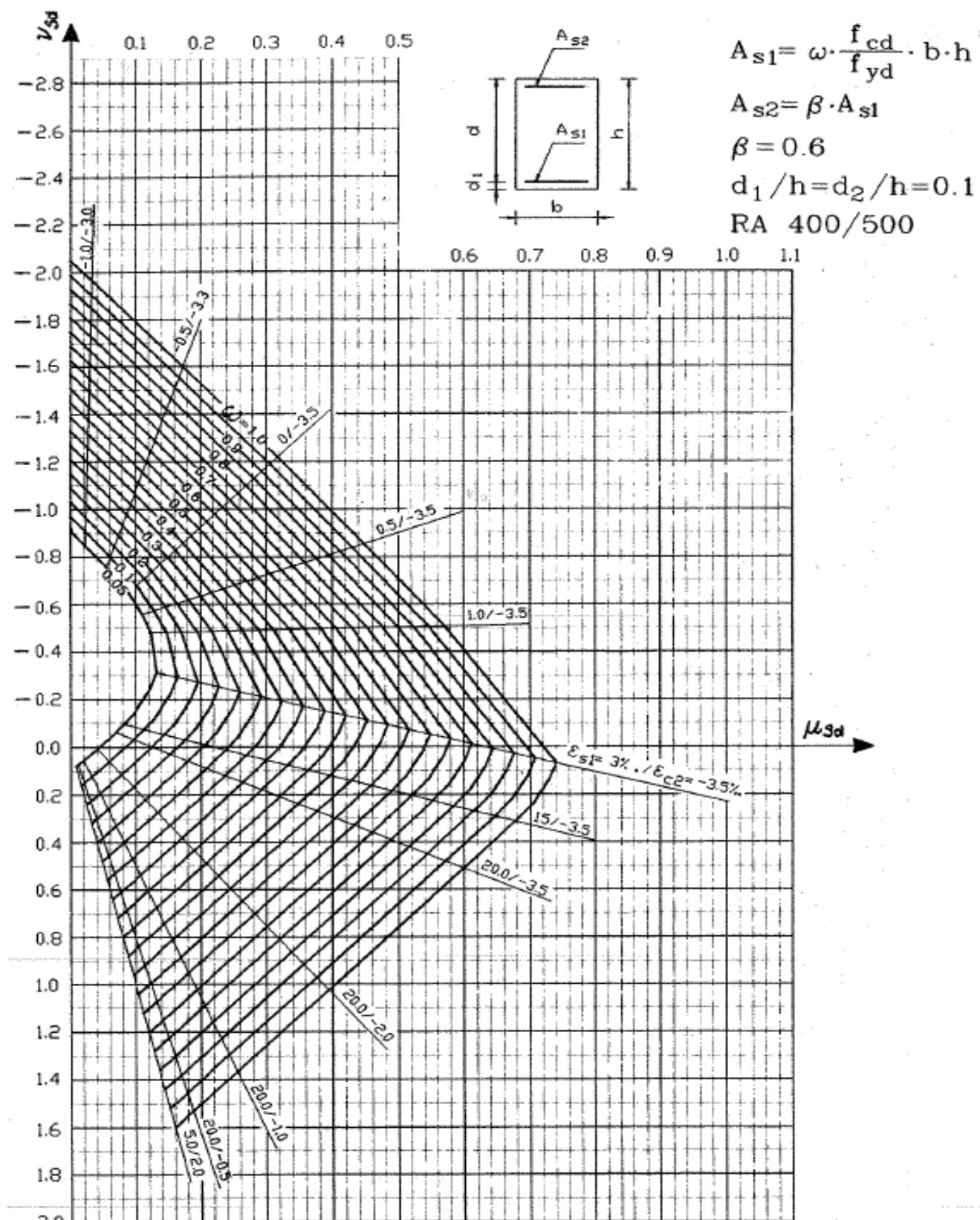
Širina grede b <sub>g</sub> (cm)	Promjer šipki armature φ (mm)								
	10	12	14	16	18	20	22	25	28
20	5	5	(5)	4	4	4	3	3	3
25	7	6	6	(6)	5	5	(5)	4	4
30	(9)	8	7	7	7	6	(6)	5	4
35	10	(10)	9	8	8	(8)	7	6	5
40	12	11	10	10	9	9	8	7	6
45	(14)	(13)	12	11	(11)	10	9	8	7
50	15	14	13	(13)	12	11	10	9	8
60	(19)	17	16	15	(15)	14	13	11	10
Profil spona φ <sub>s</sub>	d <sub>s</sub> = 6 mm				d <sub>s</sub> = 8 mm		d <sub>s</sub> = 10 mm		

Brojevi u zagradama u tabelici znače da se u grobu u jednom redu najveći može postaviti toliki broj šipki, ali se preporučuje staviti jednu manje.

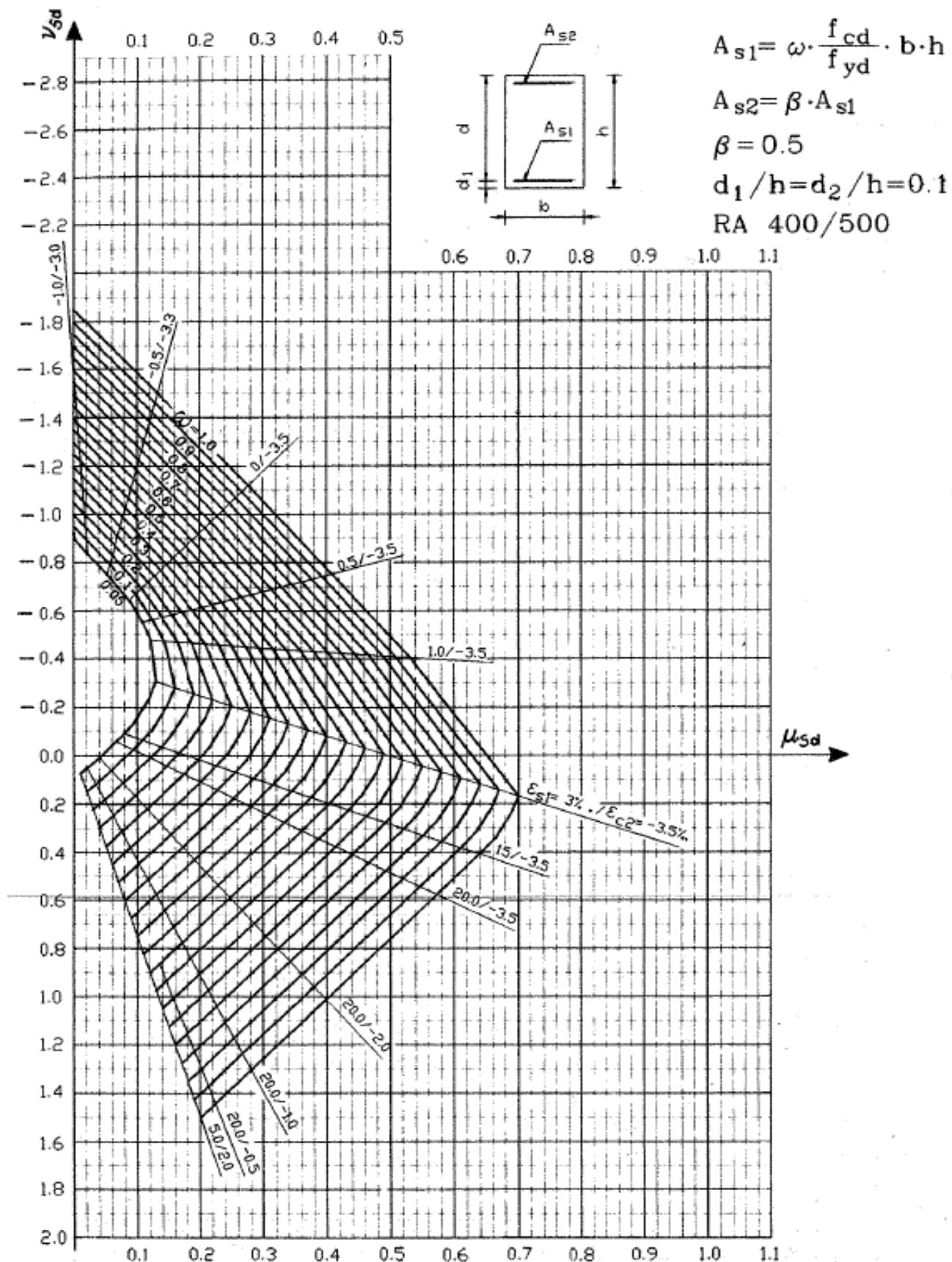
## SLIKA 1: DIJAGRAMI INTERAKCIJE

Sl. 6.20. Dijagrami interakcije - ekscentrični tlak i vlek ( $\beta=1.0$ )


 Sl. 6.21. Dijagrami interakcije - ekscentriční tlak i vlek ( $\beta=0.8$ )



Sl. 6.22. Dijagrami interakcije - ekscentrični tlak i vlek ( $\beta=0.6$ )



Sl. 6.23. Dijagrami interakcije - ekscentrični tlak i vlek ( $\beta=0.5$ )

**TABLICA 13. RAČUNSKE ČVRSTOĆE NA DJELOVANJE GLAVNIH KOSIH NAPREZANJA**

$f_{ck}/f_{ck,cuhe}$	12/15	16/20	20/25	25/30	30/37	35/45	40/50	45/55	50/60
$\tau_{Rd}$	0.18	0.22	0.26	0.30	0.34	0.37	0.41	0.44	0.48

**TABLICA 14. MINIMALNI KOEFICIJENT ARMIRANJA POPREČNOM ARMATUROM**

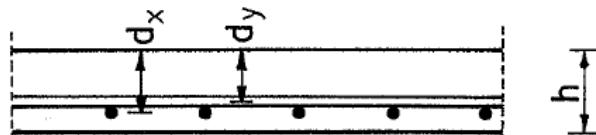
$\rho_{w,min}$	$f_{yk}$ (N/mm <sup>2</sup> )		
	220	400	500
C 12/15 i C 20/25	0.0016	0.0009	0.0007
C 25/30 i C 35/45	0.0024	0.0013	0.0011
C 40/50 i C 50/60	0.0030	0.0016	0.0013

**TABLICA 15. SREDNJA VLAČNA ČVRSTOĆA BETONA**

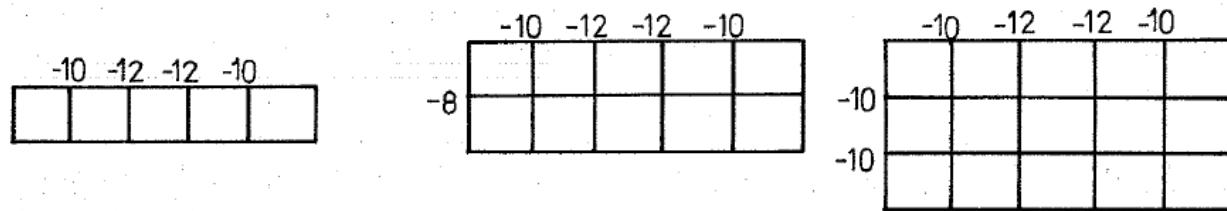
Klasa betona	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
$f_{ct,m}$	1,6	1,9	2,2	2,6	2,9	3,2	3,5	3,8	4,1

## PLOČE NOSIVE U DVA SMJERA

Armatura pravokutnih ploča računa se iz maksimalnih momenata savijanja. Pri tome treba paziti da se donji sloj armature položi u smjeru kraćeg raspona i računa sa statičkom visinom presjeka  $d_x$ , a gornji sloj s  $d_y$  (sl. 21.24).

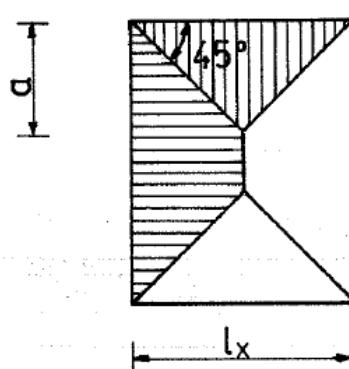


Koeficijenti "i" ispisani su na slici 21.22. ovisno o tlocrtnom položaju ležaja kao približne i zaokružene vrijednosti.



Sl. 21.22. Moguće sheme kontinuiranih ploča s pripadajućim koeficijentima za proračun ležišnih momenata savijanja

Podvlake se mogu proračunavati i sa zamjenjujućim jednolikim kontinuiranim opterećenjem:



$$\text{za kraći raspon } q_1 = 5l_x \cdot q / 16;$$

$$\text{za dulji raspon } q_2 = 0.5(1-2\alpha^2+\alpha^3)l_x \cdot q,$$

gdje je:

$$\alpha = a/l_y \approx l_x/2l_y \quad (21.27)$$

Sl. 21.23. Raspodjela opterećenja na okolne podvlake

**TABLICE 16. Tablice za proračun ploča nosivih u dva okomita smjera opterećenih ravnomjerno podijeljenim opterećenjem po cijeloj ploči**

$$\lambda = \frac{l_y}{l_x} \quad M_{x\max} = \frac{q l_x^2}{\varphi_x} \quad M_{y\max} = \frac{q l_y^2}{\varphi_y} \quad q_x = \kappa \cdot q \quad q_y = (1 - \kappa) \cdot q$$

+++++ upeti ležaj

----- slobodno položeni ležaj

Slučaj 1	$\lambda$	$\varphi_{1x}$	$\Delta$	$\varphi_{1y}$	$\Delta$	$\kappa_{1x}$	$\Delta$
	1	2	3	4	5	6	7
	0.50	169.17		10.57		0.0588	
	0.55	125.10	8.814	11.35	0.156	0.0838	0.0050
	0.60	94.94	6.031	12.30	0.190	0.1147	0.0062
	0.65	75.31	3.927	13.44	0.228	0.1515	0.0073
	0.70	61.80	2.741	14.79	0.270	0.1938	0.0084
	0.75	51.69	1.983	16.35	0.313	0.2404	0.0093
	0.80	43.97	1.544	18.01	0.331	0.2908	0.0100
	0.85	38.29	1.136	20.15	0.427	0.3430	0.0105
	0.90	34.26	0.807	22.36	0.444	0.3962	0.0106
	0.95	30.44	0.784	24.79	0.486	0.4489	0.0105
	1.00	27.43	0.602	27.43	0.527	0.5000	0.0102
	1.10	22.79	0.464	33.37	0.594	0.5942	0.0094
	1.20	19.45	0.334	40.34	0.697	0.6747	0.0080
	1.30	17.02	0.244	48.80	0.826	0.7407	0.0066
	1.40	15.22	0.180	58.45	0.985	0.7935	0.0053
	1.50	13.87	0.135	70.22	1.176	0.8351	0.0042
	1.60	12.88	0.099	84.43	1.421	0.8666	0.0033
	1.70	12.08	0.082	100.77	1.634	0.8931	0.0025
	1.80	11.45	0.061	121.89	2.082	0.9130	0.0020
	1.90	10.97	0.048	143.00	2.131	0.9287	0.0016
	2.00	10.57	0.040	169.17	2.617	0.9412	0.0012

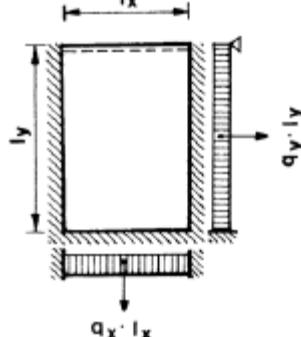
Slučaj 2	$\lambda$	$\varphi_{2x}$	$\Delta$	$\varphi_{2y}$	$\Delta$	$\kappa_{2x}$	$\Delta$
	8	9	10	11	12	13	14
	0.50	140.91		11.28		0.1351	
	0.55	107.37	6.707	12.38	0.217	0.1862	0.0102
	0.60	85.30	4.414	13.70	0.266	0.2447	0.0117
	0.65	70.59	2.942	15.29	0.320	0.3086	0.0128
	0.70	59.24	2.270	17.19	0.379	0.3751	0.0133
	0.75	50.86	1.876	19.41	0.444	0.4417	0.0133
	0.80	44.56	1.259	21.99	0.516	0.5059	0.0129
	0.85	39.70	0.971	24.96	0.595	0.5861	0.0121
	0.90	35.74	0.792	28.37	0.683	0.6212	0.0110
	0.95	32.54	0.640	32.30	0.786	0.6706	0.0099
	1.00	29.93	0.522	38.75	0.890	0.7143	0.0087
	1.10	26.02	0.391	47.58	1.083	0.7854	0.0071
	1.20	23.33	0.269	61.38	1.381	0.8383	0.0053
	1.30	21.43	0.191	78.75	1.736	0.8772	0.0039
	1.40	20.04	0.138	100.28	2.153	0.9057	0.0029
	1.50	19.02	0.103	126.84	2.636	0.9268	0.0021
	1.60	18.30	0.071	158.52	3.189	0.9425	0.0016
	1.70	17.63	0.067	196.69	3.817	0.9543	0.0012
ly UVIJEK PARALELNO S UKLIJEŠTENIM RUBOM	1.80	17.05	0.059	239.81	4.312	0.9633	0.0009
	1.90	16.67	0.037	295.08	5.527	0.9702	0.0007
	2.00	16.50	0.021	357.03	6.195	0.9756	0.0005

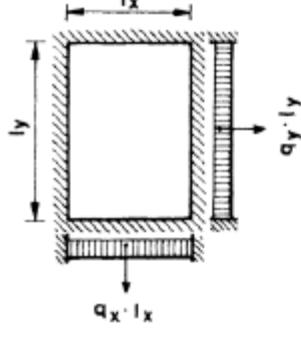
TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

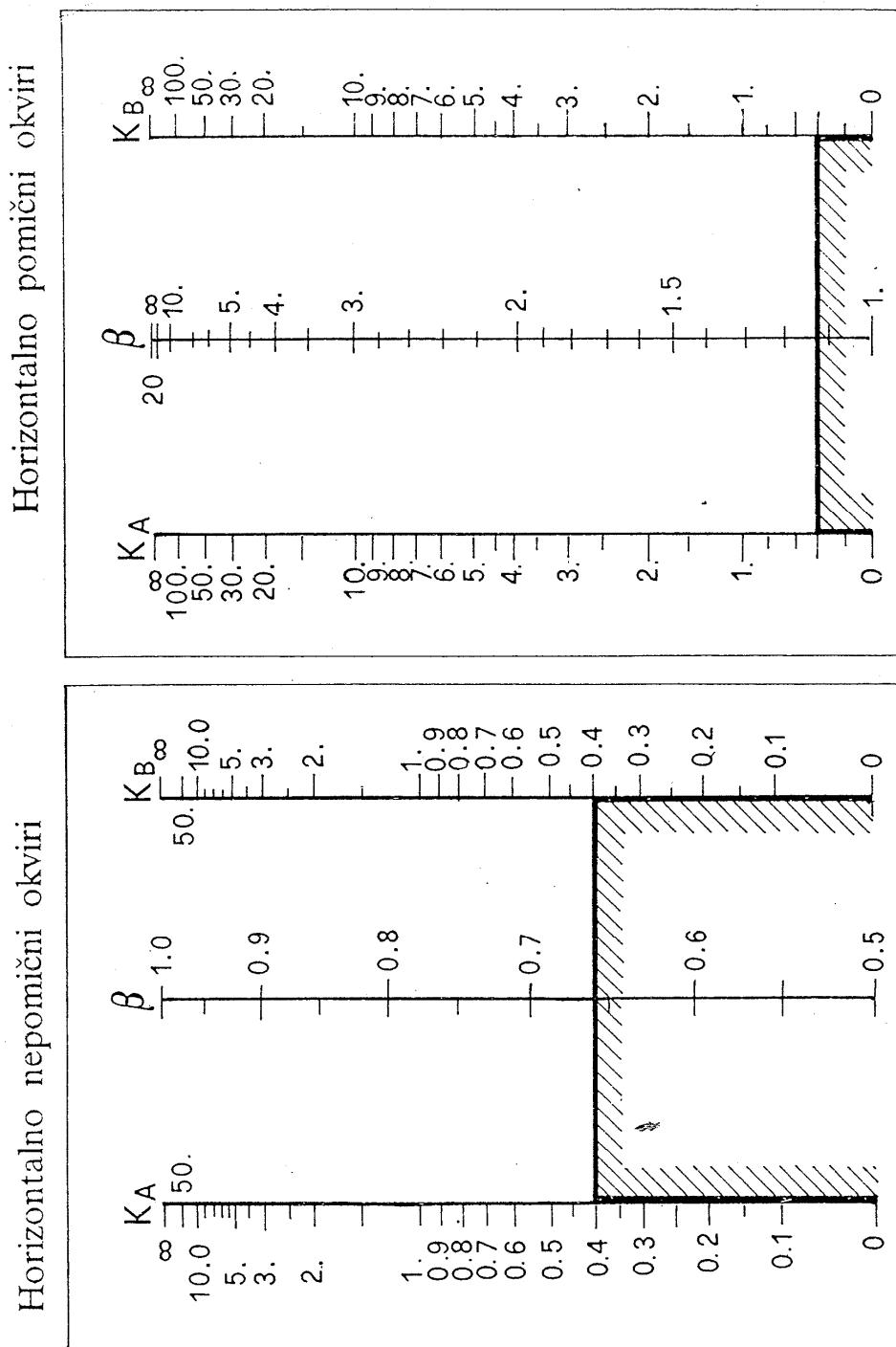
Slučaj 3	$\lambda$	$\Phi_{3x}$	$\Delta$	$\Phi_{3y}$	$\Delta$	$K_{3x}$	$\Delta$
	15	16	17	18	19	20	21
	0.50	136.06		12.48		0.2381	
	0.55	107.42	5.728	14.10	0.323	0.3139	0.0152
	0.60	87.82	3.960	16.12	0.404	0.3932	0.0159
	0.65	73.76	2.772	18.60	0.496	0.4716	0.0157
	0.70	63.69	2.014	21.16	0.603	0.5456	0.0148
	0.75	56.16	1.505	25.24	0.725	0.6127	0.0134
	0.80	50.42	1.148	29.56	0.864	0.6709	0.0116
	0.85	45.97	0.891	34.66	1.021	0.7230	0.0104
	0.90	42.48	0.698	40.65	1.199	0.7664	0.0087
	0.95	39.70	0.555	47.64	1.398	0.8029	0.0073
	1.00	37.47	0.446	55.74	1.620	0.8333	0.0061
	1.10	34.18	0.329	75.33	1.959	0.8798	0.0046
	1.20	31.93	0.225	101.68	2.635	0.9120	0.0032
	1.30	30.34	0.159	134.85	3.288	0.9346	0.0023
	1.40	29.18	0.118	175.88	4.123	0.9505	0.0016
	1.50	28.31	0.087	226.65	5.077	0.9620	0.0012
$I_y$ UVIJEK PARALELNO S UKLIJEŠTENIM RUBOM	1.60	27.64	0.067	288.36	6.171	0.9704	0.0008
	1.70	27.12	0.052	362.50	7.414	0.9786	0.0006
	1.80	26.71	0.041	450.72	8.822	0.9813	0.0005
	1.90	26.37	0.034	555.49	10.377	0.9849	0.0004
	2.00	26.09	0.028	675.81	12.132	0.9877	0.0003

Slučaj 4	$\lambda$	$\Phi_{4x}$	$\Delta$	$\Phi_{4y}$	$\Delta$	$K_{4x}$	$\Delta$
	22	23	24	25	26	27	28
	0.50	271.75		16.98		0.0588	
	0.55	194.98	6.707	17.84	0.171	0.0838	0.0050
	0.60	145.73	4.414	18.89	0.209	0.1147	0.0082
	0.65	112.92	2.942	20.16	0.254	0.1515	0.0073
	0.70	90.16	2.270	21.85	0.298	0.1938	0.0084
	0.75	73.99	1.876	23.41	0.353	0.2404	0.0093
	0.80	62.18	1.259	25.47	0.411	0.2906	0.0100
	0.85	53.34	1.768	27.84	0.465	0.3430	0.0105
	0.90	46.58	1.352	30.56	0.543	0.3962	0.0106
	0.95	41.52	1.052	33.65	0.619	0.4489	0.0105
	1.00	37.15	0.833	37.15	0.700	0.5000	0.0102
	1.10	31.09	0.606	45.52	0.837	0.5942	0.0094
	1.20	27.01	0.408	56.01	1.063	0.6747	0.0080
	1.30	24.17	0.285	69.02	1.287	0.7407	0.0066
	1.40	22.12	0.204	84.99	1.596	0.7935	0.0053
	1.50	20.62	0.150	104.38	1.940	0.8351	0.0042
	1.60	19.49	0.113	127.72	2.334	0.8676	0.0033
	1.70	18.62	0.087	155.54	2.782	0.8931	0.0025
	1.80	17.95	0.068	188.41	3.287	0.9130	0.0020
	1.90	17.41	0.053	226.93	3.852	0.9287	0.0016
	2.00	16.98	0.043	271.75	4.882	0.9412	0.0012

TABLICE I DIJAGRAMI iz predmeta BETONSKE KONSTRUKCIJE II

Slučaj 5	$\lambda$	$\Phi_{5x}$	$\Delta$	$\Phi_{5y}$	$\Delta$	$K_{5x}$	$\Delta$
	29	30	31	32	33	34	35
	0.50	246.42		17.88		0.1111	
	0.55	186.97	11.891	19.12	0.251	0.1547	0.0087
	0.60	138.61	9.671	20.68	0.313	0.2058	0.0102
	0.65	110.30	5.661	22.60	0.384	0.2631	0.0114
	0.70	90.65	3.931	24.92	0.464	0.3244	0.0123
	0.75	76.58	2.814	27.69	0.555	0.3876	0.0126
	0.80	66.24	2.068	30.98	0.658	0.4503	0.0125
	0.85	58.46	1.556	34.84	0.772	0.5108	0.0121
	0.90	52.51	1.191	39.35	0.900	0.5675	0.0114
	0.95	47.86	0.980	44.56	1.043	0.6196	0.0104
	1.00	44.18	0.736	50.57	1.201	0.6667	0.0094
	1.10	38.84	0.535	65.30	1.474	0.7454	0.0079
	1.20	35.27	0.357	84.25	1.895	0.8057	0.0060
	1.30	32.79	0.248	108.24	2.398	0.8510	0.0045
	1.40	31.01	0.178	138.11	2.987	0.8848	0.0038
	1.50	29.71	0.130	174.79	3.669	0.9101	0.0025
ly UVIJEK PARALELNO SA SLOBODNO POLOŽENIM RUBOM	1.60	28.73	0.093	219.29	4.450	0.9291	0.0019
	1.70	27.97	0.075	272.66	5.337	0.9435	0.0014
	1.80	27.38	0.059	336.02	6.337	0.9545	0.0011
	1.90	26.92	0.047	410.58	7.456	0.9631	0.0009
	2.00	26.54	0.038	497.61	8.702	0.9697	0.0007

Slučaj 6	$\lambda$	$\Phi_{6x}$	$\Delta$	$\Phi_{6y}$	$\Delta$	$K_{6x}$	$\Delta$
	36	37	38	39	40	41	42
	0.50	436.53		27.28		0.0588	
	0.55	310.15	25.276	28.38	0.220	0.0838	0.0050
	0.60	229.50	16.130	29.74	0.273	0.1147	0.0062
	0.65	175.97	10.706	31.41	0.334	0.1515	0.0073
	0.70	139.24	7.346	33.43	0.404	0.1936	0.0084
	0.75	113.30	5.189	35.85	0.483	0.2404	0.0093
	0.80	94.51	3.757	38.71	0.573	0.2906	0.0100
	0.85	80.60	2.782	42.08	0.673	0.3430	0.0105
	0.90	70.10	2.100	46.00	0.784	0.3962	0.0108
	0.95	62.04	1.614	50.53	0.907	0.4489	0.0105
	1.00	55.74	1.259	55.74	1.043	0.5000	0.0102
	1.10	46.77	0.897	68.48	1.274	0.5942	0.0094
	1.20	40.90	0.588	84.80	1.632	0.6747	0.0080
	1.30	36.89	0.400	105.38	2.057	0.7407	0.0066
	1.40	34.08	0.282	130.92	2.555	0.7935	0.0053
	1.50	32.04	0.204	162.22	3.130	0.8351	0.0042
	1.60	30.54	0.151	200.13	3.790	0.8676	0.0033
	1.70	29.40	0.114	245.53	4.540	0.8931	0.0025
	1.80	28.52	0.088	299.38	5.385	0.9130	0.0020
	1.90	27.75	0.077	362.69	6.331	0.9287	0.0016
	2.00	27.28	0.047	436.53	7.384	0.9412	0.0012

**JACKSONOVI I MORELANDOVI NOMOGRAMI**

Sl. 11.9. Jacksonovi i Morelandovi nomogrami